SACRAMENTO RIVER TOPOGRAPHIC AND BATHYMETRIC DATA ACQUISITION TECHNICAL REPORT

US Bureau of Reclamation Annual Funding Agreement R23AV00012 Task 20

Prepared for:

Rod Wittler and Jenna Paul Bureau of Reclamation Bay Delta Area Office, CVPIA Division Sacramento, CA

Prepared by:

Yurok Tribe Fisheries Department Technical Services Program 5435 Ericson Way, Suite 1 Arcata, CA 95521

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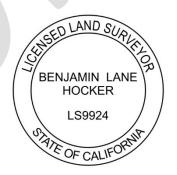
EXECUTIVE SUMMARY

The Yurok Tribe Fisheries Department Design and Technical Services Program (YT TSP) was tasked by the United States Bureau of Reclamation (USBR) Central Valley Project Improvement Act (CVPIA) Habitat Restoration Program (HRP) to conduct a system wide base-line data collection effort across 174 miles on the Sacramento River to develop habitat and decision support modeling. High resolution aerial imagery, topography, bathymetry, and water surface elevations were provided to the USBR and are intended to support the development of a hydrodynamic model framework for predicting habitat across a range of flows. The model will support measures needed to restore anadromous fish to optimum and sustainable levels in accordance with the restored carrying capacities of Central Valley rivers, streams, and riparian habitats. The data will also be used to support planning, design, and analysis of current and future restoration projects on the Sacramento River.

This report summarizes the data collection techniques and procedures as well as the processing and data integration methodologies used to develop the terrain model.

SURVEYOR'S STATEMENT

I, Benjamin Lane Hocker, as a licensed land surveyor in the state of California (No. 9924) certify that the imagery and LiDAR data sets were compiled by the Yurok Tribe's Fisheries Department Design and Technical Services Program and tested to meet ASPRS Positional Accuracy Standards for Digital Geospatial Data, Edition 2 (2023). The imagery was tested for a 10 cm RMSE_H horizontal positional accuracy class. Actual horizontal accuracy was found to be RMSE_H = 8.9 cm. The LiDAR was tested for a 10 cm RMSE_V vertical accuracy class. NVA accuracy was found to be RMSE_V = 5.4 cm.



Benjamin Lane Hocker, CA PLS 9924 July 12, 2024

The data collected for this project are intended for purposes of topographic and bathymetric modelling and visualization only, and not for boundary determination or depiction. Nor are these data, nor any products derived therefrom intended to be used for marine navigation. All information provided represents the results of the surveys on the dates collected and can only be considered as an indication of the general conditions existing at the time.

INTRODUCTION

Developing a seamless digital terrain model (DTM) of the project area, as shown in Figure 1, required multiple technologies to efficiently collect the required data. An airborne topobathymetric lidar survey was proposed to first collect data in the upland and wetted areas to extinction, where multibeam sonar would then be used to fill in the gaps. Bathymetric lidar is an extremely fast and efficient technology, but penetration is dependent on factors such as water clarity and riverbed reflectivity. It was expected that the conditions in the Sacramento River would result in a penetration of approximately 2 meters in most circumstances. In areas deeper than this at the time of the survey, multibeam sonar would then be used to complete the model. Vessel based multibeam sonar surveys are a much slower method, becoming more inefficient in shallow water, but would not be limited by depths or turbidity in the river. As such, an approach was designed that leveraged the advantages of each technology.

The original project plan specified the airborne topo-bathymetric lidar survey would have been completed first at the lowest possible flow (lowest water levels) at the start of the year to maximize coverage then processed to develop preliminary data extents. The multibeam sonar survey would then follow, with the lidar coverage used in real-time to navigate the survey boats, filling in data only where required. Unfortunately, the spring of 2023 saw unexpected, extensive flooding in the area. Water levels and turbidity were extremely high, negating the possibility of a successful topo-bathymetric lidar survey campaign.

YT TSP mobilized the bathymetric lidar aircraft and equipment to the area but due to the river conditions, it remained on standby for the month of March. To keep the project on track, YT TSP in consultation with the USBR elected to collect the multibeam sonar first, using the higher water levels to maximize sonar coverage. The lidar was then flown at low water in the summer/fall of 2023. Completing the surveys in this order was not the most efficient approach, as it led to considerably more overlap in some areas to reduce the risk of gaps between the datasets.

Airborne imagery was collected over a larger Area of Interest (AOI) as shown in Figure 1. The imagery AOI was collected during dedicated flights, tailored to the best times of day for sun angle. Imagery was also co-collected in the lidar AOI during the topo-bathymetric flights. These flights prioritized lidar data collection, and the generated orthoimagery were only used to colorize the lidar point clouds and assist in editing.

Water surface profiles were also collected along the Sacramento River, Clear Creek and Battle Creek. The collections were to target a high flow and low flow condition for the purpose of hydrodynamic modelling.

YT TSP partnered with Marker Offshore, LLC and Hexagon, Inc. to build capacity and provide additional equipment necessary for a project of this size. Marker Offshore provided additional resources to assist with the vessel-based surveys along the Sacramento River while Hexagon provided the topo-bathymetric lidar sensor, operator and data processing support.

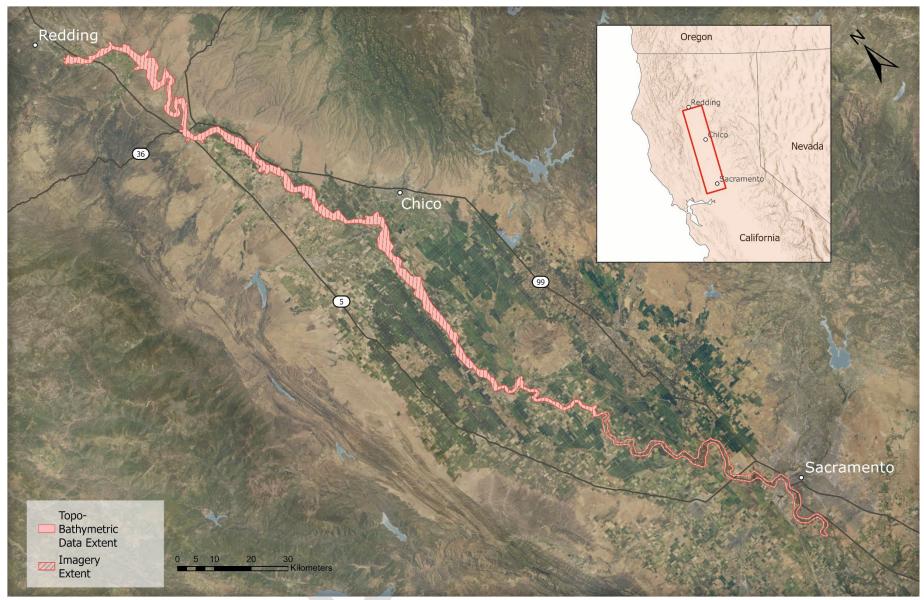


Figure 1: Project Location and Survey Extents

The amended schedule to comply with the scope of work included:

- Coordinating with USBR staff and project partners on timing of sonar and lidar surveys,
- Establishing and verifying a control network for the project,
- Completing multibeam sonar surveys during high water periods in the spring of 2023,
- Completing topo-bathymetric lidar surveys during the low water period in the fall of 2023,
- Completing high resolution airborne digital imagery surveys in the fall of 2023,
- Collecting accuracy assessment data,
- Collecting water surface data during high and low flow periods,
- Integration of data to construct a seamless digital terrain model, and
- Developing data deliverables and products.

Geodetic Control

The horizontal datum for this project is based on NAD83 (2011) Epoch 2010.00 with coordinates projected to UTM zone 10 North. The vertical datum is based on NAVD88 using GEOID18. Lineal units of meters with all times relative to UTC.

National Geodetic Survey (NGS) control points, Continually Operating Reference Stations (CORS) and the California Surveying and Drafting Supply, Inc.'s Real Time Network (CSDS RTN) were used to control the project. Prior to data collection, a control survey was completed to establish or verify coordinates for points to be used on the project. This work and the results are documented in the accompanying *Sacramento River Topographic and Bathymetric Data Acquisition Control Survey Report, Yurok Tribe Fisheries Department, July, 2024.* From this report, the coordinates held for the project are presented in the following tables: CSDS RTN stations in Table 1 and published NGS control points in Table 2.

Station ID	Northing (m)	Easting (m)	Elevation (m)	Comment
CH1G	4401895.184	597057.674	62.312	Adjusted CSDS
OR1K	4373844.263	624441.360	68.467	Adjusted CSDS
RD1L	4487051.663	560656.072	160.510	Adjusted CSDS
SACR (DH8725)	4279776.703	643204.811	37.939	NGS CORS - HELD
WD1J	4281430.019	607210.869	31.267	Adjusted CSDS
WI1H	4334394.239	573472.349	31.733	Adjusted CSDS
YC1I	4333788.301	617298.868	25.802	Adjusted CSDS
LD1K	4222467.086	653009.529	24.380	Adjusted CSDS

Table 1: CSDS RTN Stations

Table 2: NGS C	able 2: NGS Control Points			
PID	Northing (m)	Easting (m)	Elevation (m)	Comment
DH6394	4487029.808	555168.142	139.898	NGS Published
DH6520	4344671.695	583591.357	17.765	NGS Published
DH6521	4322066.682	600660.298	18.460	NGS Published
DH6625	4417762.035	575711.774	65.803	NGS Published
DL9132	4474357.404	568199.098	115.746	NGS Published
DL9142	4457255.726	566217.047	97.167	NGS Published
DL9190	4387157.618	585967.041	37.020	NGS Published
DL9193	4312289.141	608034.859	8.584	NGS Published
KS2014	4333250.412	594909.156	12.601	NGS Published
KT0518	4367972.786	584517.810	27.305	NGS Published
KT1807	4399844.111	583915.690	47.886	NGS Published
LU2291	4445793.944	566086.713	95.350	NGS Published
AC9219	4262862.915	623642.507	9.805	NGS Published
AI5062	4294557.490	610582.482	13.757	NGS Published
AI5069	4278610.814	624246.418	11.904	NGS Published
DL9193	4312289.134	608034.857	8.546	NGS Published
DN4101	4252253.230	632347.444	12.144	NGS Published

Table 2: NGS Control Points

Additional temporary control points set during the project are shown in Table 3.

Table 3: Additional Temporary Control Points

Station ID	Northing (m)	Easting (m)	Elevation (m)	Comment
RBL1	4445479.853	563764.666	103.190	Lidar base station
CC1	4483946.586	550488.206	141.408	Clear Creek float

For additional information on acquisition, processing and accuracy for the control network, please refer to the control report.

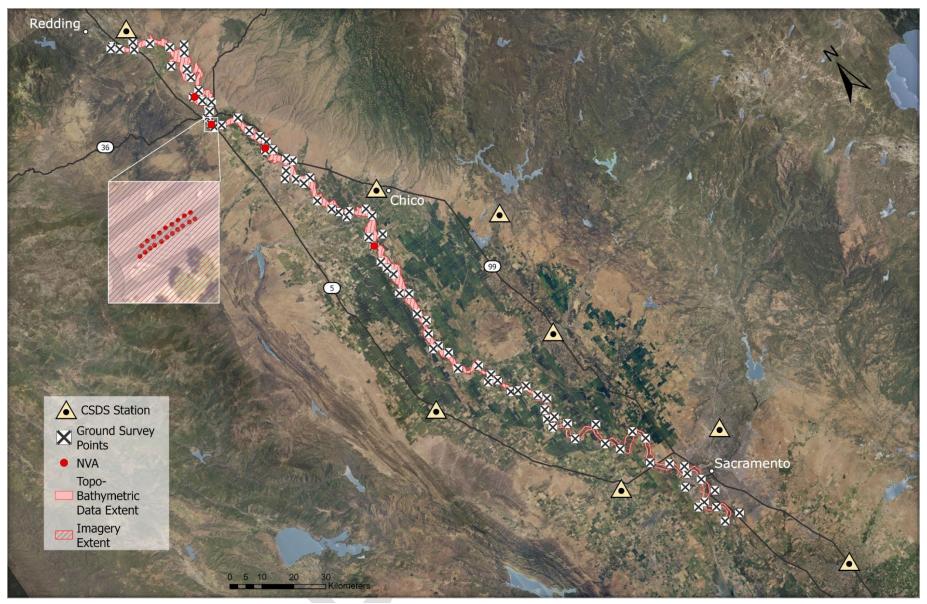


Figure 2: Reference Stations and Ground Survey Points

DATA ACQUISITION

Data acquisition began with the control survey then proceeded to multibeam once the flood levels receded and boat ramps became accessible. Airborne lidar and imagery then followed during low water. A summary of the data collected by dates is shown in Table 4.

Table 4: Acquisition Dates	
Data Type	Acquisition Dates
Ground Surveys	3/31/2023 - 5/02/2024
Multibeam Sonar	4/29/2023 - 7/30/2023
Topo-Bathymetric Lidar	9/22/2023 - 10/03/2023
Ortho Imagery	9/05/2023 - 9/07/2023
Sacramento R. High Flow Water Surface Profile	4/25/2023 - 4/28/2023
Sacramento R. Low Flow Water Surface Profile	9/22/2023 - 10/03/2023
Clear Creek Water Surface Profile	5/02/2023
Battle Creek Water Surface Profile	10/11/2023

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Ground Survey

Ground survey points (GSPs) were established to perform quality assurance checks on the lidar and imagery data. A ground survey plan that evenly distributed points throughout the lidar and imagery AOIs was developed and executed by the YT TSP prior to airborne data collection. GSPs were collected using the Global Navigation Satellite System (GNSS) real time kinematic (RTK) survey techniques with augmentation from the CSDS RTN. Points were established using a Trimble R10 or R12 receiver with observations of at least 180-epochs. The receiver was then reinitialized, and the point was remeasured as an independent check. All GSP measurements were made during periods with a Position Dilution of Precision (PDOP) of ≤ 3.0 with at least six satellites in view of the rover. Relative errors for any GSP position were limited to less than 0.03 m horizontally and 0.05 m vertically, evaluated at the 95% confidence level.

To ensure proper rover configuration and system performance, checks were made to control points documented in the prior control survey.

GSPs consisted of existing high visibility road markings and painted air targets (PATs) located on hard, flat surfaces. These points served as horizontal and vertical reference points. Non-Vegetated Vertical Accuracy (NVA) points were also collected to serve as vertical checks only. Survey control, GSP and NVA locations are shown in Figure 2. A total of 95 93 GSPs and 159 NVA points were collected for this project to control and validate the accuracy of the lidar and imagery datasets.

Airborne Topo-Bathymetric Lidar

Lidar surveys were accomplished using a Leica Chiroptera 4X (CH4X) topographic and bathymetric sensor installed in a Leica PAV100 gyro-stabilized mount. The system was deployed in the Yurok Tribe's Cessna Caravan 208 EX (N901CA) aircraft. Table 5 summarizes the settings used to meet the project specifications.

Table 5: Lidar Parameters				
Lidar Survey Settings & Specifications				
Aircraft	Cessna Caravan 208 EX (N901CA)			
Sensor	Leica Chiroptera 4X			
Resolution/Density (Topographic)	~10 pts/m ² /swath			
Survey Altitude (AGL)	~1,300 ft			
Survey speed	~130 knots			
Swath Width	~380 m			
Swath Overlap	15%			
Number of Flight Lines	175			
Total Line Length	3292 km			

Table 5: Lidar Parameters

The CH4X is an integrated system including a topographic laser, bathymetric laser, 4-band Leica RCD30 digital camera and inertial navigation system (INS). The digital imagery from the CH4X flights was primarily used to colorize the lidar point cloud and assist in data editing.

Flightlines were planned in Leica MissionPro software and are presented in Figure 3. The survey area was broken up into 8 manageable blocks (A through H from north to south), where flightlines were designed to minimize the time in turns while covering the required AOI. The sinuous nature of the river lead to multiple blocks and directions since the speed of the aircraft mandates straight lines.

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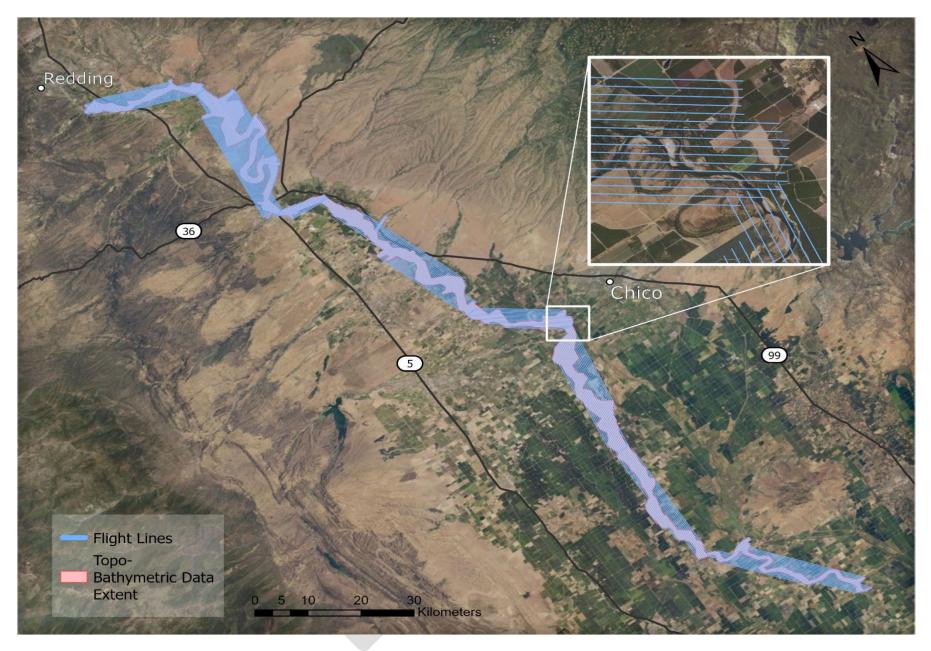


Figure 3: Topo-Bathymetric Data Flight Lines

Multibeam Sonar

Two boats were deployed on the river surveying simultaneously along different reaches in order to meet project deadlines. The *GMA-Jet*, a 19-foot aluminum river sled operated by YT TSP, and the *Ospika*, a 21-foot aluminum workboat operated by Marker Offshore, shown in Figure 4.



Figure 4: Survey Vessels (GMA-Jet left, Ospika right)

Each vessel used a NORBIT Compact Wideband Multibeam Sonar with integrated Applanix POS/MV inertial navigation system (iWBMSh). The sonars were configured to operate at 400 kHz and generate 256 beams for each ping. Equi-distant mode was used in the main channel, and equi-angular mode was used along the banks where the sonar swath was rotated to obtain coverage up to the waterline where possible. Swath angle was adjusted in real-time to maximize data quality and coverage.

Surface sound speed was recorded using the NORBIT integrated sound velocity probe and profiles were taken using AML BaseX2 on the *Ospika* and a Valeport Swift sound speed profiler on the *GMA-Jet*. Raw data were logged using QPS Qinsy software which also provided real-time vessel navigation.

The AOI was divided up into 87 roughly 2-mile long sections named according to the approximate river mile (RM). This created a consistent naming convention and allowed the vessels to focus on smaller areas at a time, ensuring they were complete before moving on. The multibeam surveys were completed over a total of 84 boat days (43 for the *GMA-Jet* and 41 for the *Ospika*) spanning 56 calendar days.

Prior to data acquisition, each vessel's sensor offsets were measured and entered into the survey systems. A patch test was then completed in the Keswick Reservoir to determine the sonar mounting angles relative to the IMU reference frame. Results are presented in Table 6 and Table 7 for the two vessels. Latency for both systems was verified to be zero seconds.

The dynamic draft (settlement) curve for the *Ospika* was also measured in the Keswick Reservoir during patch testing. The vessel was operated at a fixed engine RPM for a 1-minute duration,

and the elevation change measured with each RPM using the post processed vessel trajectory. A table of settlement vs. RPM/Speed was then developed and is presented in Figure 5.

Sensor	X (+Forward) m	Y (+Starboard) m	Z (+Down) m
Primary GNSS L1 PC	-0.783	0.000	-2.265
Secondary GNSS L1 PC	1.223	-0.008	-2.271
IMU Reference Point	0.255	0.000	0.079
Sonar Reference Point	0.000	0.000	0.000
Reference Frame	Roll °	Pitch °	Yaw °
Sonar to IMU	-0.116	-0.001	-0.116

Table 6: GMA-Jet Offsets

Table 7: Ospika Offsets

Sensor	X (+Forward) m	Y (+Starboard) m	Z (+Down) m
Primary GNSS L1 PC	-2.309	0.508	-2.180
Secondary GNSS L1 PC	-0.310	0.501	-2.126
IMU Reference Point	0.255	0.000	0.079
Sonar Reference Point	0.000	0.000	0.000
Reference Frame	Roll °	Pitch °	Yaw °
Sonar to IMU	-1.030	-0.710	0.000

RPM	SPEED (M/S)	SETTLEMENT	0.00
1000	1.65	-0.009	-0.00
1200	1.93	-0.012	-0.01
1400	2.21	-0.016	E -0.01
1600	2.45	-0.020	-0.02
1800	2.63	-0.023	20.0- (m) 20.0- went (m) 20.0- settlewent (m)
2000	2.80	-0.027	-0.03
2200	2.92	-0.029	-0.03
2400	3.05	-0.032	-0.04
2600	3.23	-0.036	

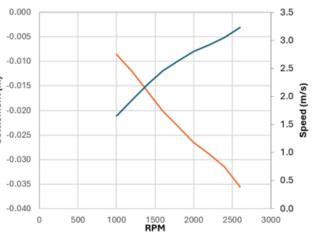


Figure 5: Ospika Dynamic Draft Table

Multibeam data were collected by the survey vessels up to approximately 2 meters water depth (at high flow). This was estimated as a conservative point where sufficient overlap would be reached with the lidar. This was not the most efficient order in which to collect the data, but unseasonably high-water levels early in the year forced the team to collect sonar before lidar.

Airborne Imagery

Digital imagery was acquired using a PhaseOne PAS 280i system installed in a SOMAG GSM 4000 gyro-stabilized mount. The PhaseOne PAS 280i is a large format nadir digital imaging system. The system consists of an iXM-RS280F metric camera that utilizes two 90mm Rodenstock lenses and an iXM-RS150F NIR that utilizes a single 150mm lens. The system simultaneously collects RGB and NIR imagery, creating a 280 MP 4-band image solution. The imaging specifications and project specific settings are shown in Table 8. Topographic Lidar was simultaneously collected during imagery acquisition with a Riegl VQ-1560 II-S installed in a SOMAG GSM 40000 gyro-stabilized mount. The additional lidar was co-collected to orthorectify the imagery; this was necessary due to the differing topo-bathymetric lidar and imagery extents. Flightlines were planned in TopoFlight Mission Planner software and are presented in Figure 6.

Digital Orthophotography Specifications			
Aircraft Used	Cessna Caravan 208 EX (N901CA)		
Sensor	PhaseOne PAS 280i		
Spectral Bands	Red, Green, Blue, NIR		
Pixel Size	3.76 µm		
Focal Length RGB	90 mm		
Focal Length NIR	50 mm		
Field of View RGB	32.9° along track X 45.7° across track		
Field of View NIR	42.2° along track X 56.2° across track		
Cross Track Overlap	>55%		
Along Track Overlap	60%		
Resolution Ground Sample Distance	<10cm		
Flight Altitude (AGL)	~2377m		
Data Format	16-bit IIQ		

Table 8: Imagery Specifications and Settings

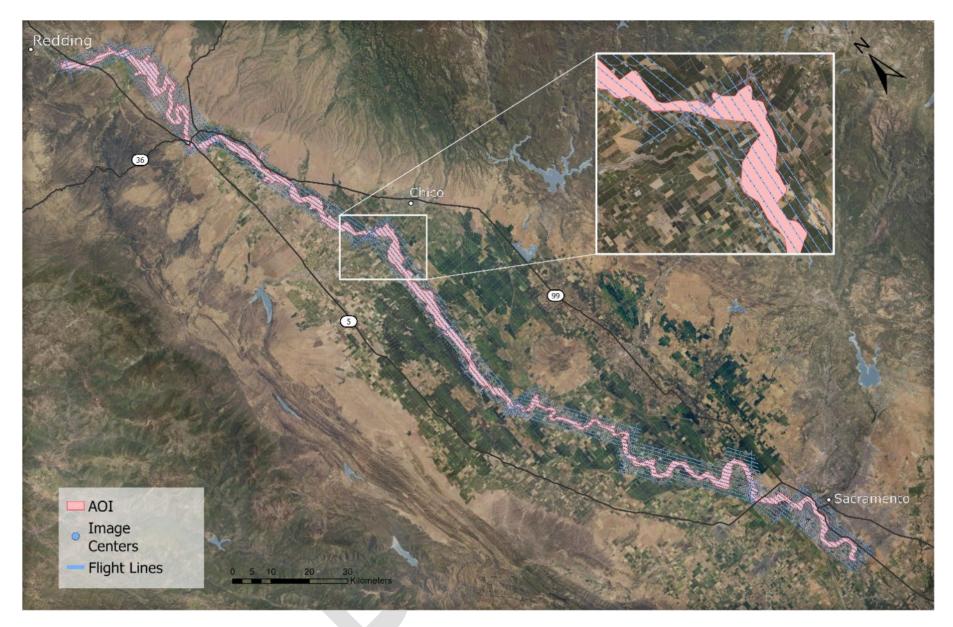


Figure 6: Imagery Flight Lines

Water Surface Profiles

Water surface profiles were measured at multiple flows along the Sacramento River, Clear Creek and Battleground Creek for modelling purposes. The statement of work required the following:

- Sacramento River: From the Clear Creek confluence to Wilkins Slough (roughly 174 miles) at high and low flows defined by:
 - High flow discharge: approximately 10-15,000 cfs at upstream end of the reach based on gauge records at Keswick Dam (USGS 11370500) and Bend Bridge (USGS 11377100); and 15-25,000 cfs at the downstream end of the reach, Sacramento River below Colusa (USGS 11389500)
 - Low flow discharge: approximately 3,250 cfs at upstream end of the reach based on gauge records from Keswick Dam (USGS 11370500) and Bend Bridge (USGS 11377100); and 5,250 cfs at the downstream end of the reach, Sacramento River below Colusa (USGS 11389500)
- Clear Creek: Lower 2 miles to the confluence with the Sacramento River
- Battle Creek: Lower 6 to the confluence with the Sacramento River

A combination of techniques was used as summarized in Table 9 along with the average and standard deviation of the USGS observed discharge over the collection period. The high flow profile along the 174-mile Sacramento River reach was completed by the *Ospika* while collecting initial multibeam depths in the deeper sections of the river. This single pass required 4 days due to the relatively slow vessel speeds (typically between 7-8 knots).

The low flow on the Sacramento River and the Battle Creek profiles were extracted from the topo-bathy lidar derived water surface. This method was verified by comparison to a separate vessel float on Battle Creek as further discussed in the data processing section.

The Clear Creek water surface profile was measured with a GNSS mounted to a kayak that was floated downstream. The GNSS was configured to not only receive real-time corrections from a base station established on control point CC1, but also log raw observables internally at 1-second intervals.

Water Surface Profile	Method	Reported Discharge		
Sacramento River High Flow	Vessel float	Keswick Dam: Average = $8,690 \text{ cfs}$ ($\sigma = 70 \text{ cfs}$)Bend Bridge: Average = $12,330 \text{ cfs}$ ($\sigma = 240 \text{ cfs}$)Below Colusa: $13,070 (\sigma = 550 \text{ cfs})$		
Sacramento River Low Flow	Lidar water surface	Keswick Dam: Average = $6,670 \text{ cfs} (\sigma = 180 \text{ cfs})$ Bend Bridge: Average = $7,610 \text{ cfs} (\sigma = 280 \text{ cfs})$ Below Colusa: $6,230 (\sigma = 240 \text{ cfs})$		
Clear Creek	Vessel float	245 cfs at Clear Creek near Igo (USGS 11372000)		
Battle Creek	Lidar water surface with vessel float check	235 cfs at Battle Creek below Coleman (USGS 11376550)		

Table 9: Water Surface Profile Summary

The high flow discharge water surface profile on the Sacramento River was collected on days that fell slightly below the targets. This was due to adjustments in the schedule as a result of the flooding earlier in the spring.

Targets were also not met on the low flow water surface acquisition. To meet project deadlines, the lidar data collection was completed at the earliest low flow on the Sacramento. This time period had flows that were slightly higher than the targets but were nearly the lowest over the entire year. Targeted flows of 3,250 cfs were not ever reached during 2023 as shown by the plot of discharge at Bend Bridge in Figure 7.

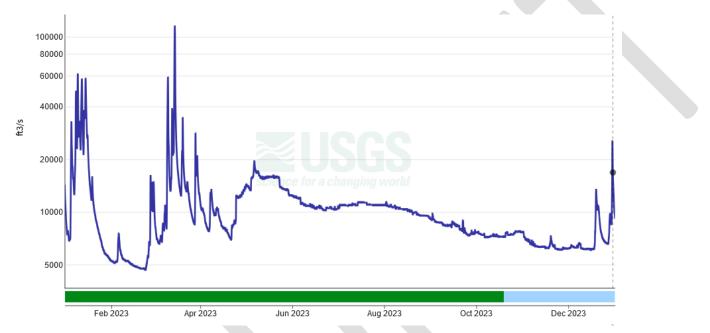


Figure 7: 2023 Discharge at Bend Bridge near Red Bluff (USGS 11377100)

DATA PROCESSING

Ground Survey

Ground surveys were processed by YT TSP staff in Trimble Business Center version 2023.11 software. Raw data from the survey controllers were imported then geodetic settings, antenna heights and models were verified. Accuracies were reviewed to ensure project objectives were met and any points falling below thresholds were removed.

Position checks were reviewed to ensure control network accuracy as well as proper base station and rover configuration. Results are presented in Appendix A.

The final points were then exported in text format for lidar and imagery product accuracy assessment. Points were also merged with photos and exported in report format, provided in Appendix C.

Topo-Bathymetric Lidar Data

Topo-bathymetric lidar data were initially processed in the field immediately after acquisition. This was done to develop coverage maps and validate that the data collected would meet project specifications. Final processing to a partially classified point cloud was completed by Hexagon then delivered to YT TSP for integration.

To begin the initial field processing, the aircraft trajectory was determined. The CH4X contains a Novatel SPAN Inertial Navigation System (INS) with a Novatel LCI IMU. The GNSS receives data on GPS L1, L2, L2C, L5, and GLONASS L1, L2 signals. Trajectory processing combines data from the sensor integrated GNSS and IMU with data from the project base station to compute accurate position and orientation of the sensor during the flight. Novatel Inertial Explorer was used to combine the GNSS and IMU processing in a tightly coupled solutions for high accuracy Differential GNSS/IMU post-processing solutions.

The DGNSS/INS solutions file was combined with the sensor calibration values and raw sensor data in Leica Survey Studio (LSS) to produce georeferenced LAS unclassified point clouds. A calibration flight was completed prior to the project to determine the relative orientation between the scanner, INS and GNSS systems. This calibration is applied to the raw sensor data, in combination with the DGNSS/INS solution file to produce the georeferenced LAS v1.4 unclassified points clouds.

Once lidar data were verified, tiles were generated and imported into TerraSolid software in a 500m x 500m tiling scheme.

Initial classification of the LAS point cloud was then completed by Hexagon using TerraSolid TerraScan software. The LSS automated classifications were reviewed and improved using additional macros. After these automated routines were complete, technicians manually inspected an editable TIN model for each respective tile to further correct any misclassification of points. After ground classification is complete, a combination of automated classification macros and manual editing by technicians were used to classify the bathymetric points. The resulting classified tiles were delivered to YT TSP on a series of hard drives for final processing and integration.

YT TSP reviewed the delivered data and completed additional processing using TerraScan version 23.017 and LAStools 2024.01 software to enhance the ground classification. Initial ground points were filtered then used as a seed surface to increase detail and density, particularly in areas of vegetation where multiple lidar returns were present. Further processing was done to clean up the ground classification along the banks and also remove noise throughout the dataset.

Multibeam Sonar Data

Once sonar data collection was complete, YT TSP and Marker Offshore processed data from their respective boats in QPS Qimera version 2.5.3 software. Processing at this stage was segmented into the roughly 2-mile long sections of the river the data were collected in.

To enhance real-time positioning and orientation accuracies, raw data from the POS/MV were post-processed in Applanix POSPac MMS version 8.9 software. GNSS reference station data were used from existing NGS CORS and CSDS stations using Applanix SmartBase methodology. Applanix SmartBase uses a network of reference stations to estimate atmospheric, orbital and clock errors which are then used to correct for errors at the rover location at each epoch. This post-processing method is applying the Virtual Reference Station (VRS) concept to extend the maximum baseline lengths for integer ambiguity resolution.

The resulting Smoothed Best Estimate of Trajectory (SBET) files were applied to georeference the raw soundings in Qimera. Sound speed profiles were then applied, then preliminary surfaces developed. A combination of manual and automated filtering and editing techniques were then employed to remove noise from the dataset.

The cleaned Qimera QPD format files from both boats were then combined over the entire AOI into one large project and a final gridded surface was generated at 1 meter resolution using Combined Uncertainty and Bathymetry Estimator (CUBE) algorithms. A single project wide surface was developed to eliminate any edge effects that may have been introduced by gridding the smaller sections individually.

Digital Imagery

Digital imagery was processed in iXProcess version 1.2 and went through multiple processing steps. Images were color balanced, levels adjusted, and finally corrected for geometric distortion. The RGB and NIR images were then combined into CIR format and exported from PhaseOne IIQ format to industry standard tiff format. An image exterior orientation file including position and orientation for each image was produced using the smoothed best estimate of trajectory (SBET) produced in POSPac MMS version 8.9 using SmartBase methodologies. The images were then imported into Agisoft's Metashape version 2.1.1 photogrammetric software where analytical aerial triangulation was performed using ground control, automatically generated tie points, and camera calibration information.

Finally, aligned images were orthorectified using the co-collected lidar-derived ground model to remove displacement effects from topographic relief inherent in the imagery. The resulting orthorectified image frames were mosaicked, and seamlines were reviewed and edited as needed.

Water Surface Profiles

Several techniques were used to collect the water surface profiles throughout the project, depending on the timing of the acquisition and the size of the river. The following sections outline the processing steps for each to generate an export file with date, time, northing, easting and water surface elevation. These data were then viewed and edited in a continuous profile in Caris HIPS version 10.4.20 software. The final data were then exported to separate ESRI 3D point feature SHP files.

Clear Creek

Raw GNSS data collected using the kayak float were processed in Trimble Business Center (TBC) version 2023.11 software. Data from the real time solution using RTK had several gaps and areas of poor accuracy since the telemetry from the base station was occasionally blocked by the topography. The raw observables from the base and rover were used in a Post-Processed Kinematic (PPK) solution to generate accurate positions. The antenna height above water was applied to the solution then the values were manually reviewed and edited to create a single continuous track down river.

Battle Creek

Initially, the Battle Creek profile was collected using the same methodology as Clear Creek, but there were significant gaps in both the RTK and PPK profiles due to dense overhead vegetation. To develop a continuous profile, the derived water surface points from the bathymetric lidar were used. The derived water surface is a set of synthetic points created in LSS during raw bathymetric lidar data processing. These points are generated from topographic laser channel of the system and used as the surface to begin refraction correction of the bathymetric laser data.

A rough centerline was digitized along Battle Creek which was then used to extract the derived water surface points from the bathymetric lidar data set. The extracted points were then gridded and thinned to a 5 m spacing. The thinned dataset was then reviewed and edited for erroneous data. To validate the accuracy of this method, data along Battle Creek collected with a GNSS in open areas with good accuracy were compared to the derived water surface points. The average difference between the two methods was computed to be 0.03m with a standard deviation of 0.02m in the lower section of the creek.

Sacramento River High Flow

Data from the post-processed vessel trajectory (as discussed in the multibeam processing section of this report) were corrected to the water surface by both a static and dynamic draft. Static draft is the height of the vessel reference point above or below the water surface measured at the start and end of the survey day to account for changes in loading due to fuel burn. Dynamic draft (or settlement) is the change in the height of the reference point of vessel relative to the water surface due to the hull speed. As settlement is a function of speed through the water, it is difficult to determine in a dynamic current environment. It is best approximated by the engine RPM, which was documented during the collection of the water surface profiles. The dynamic draft correction was applied to the elevation data using a table of RPM vs. dynamic draft correction developed during the initial calibrations in Keswick Reservoir at the start of the project.

Sacramento River Low Flow

The low flow profile was extracted from the bathymetric lidar derived water surface points in a similar manner to Battle Creek. Sacramento River lidar data were collected over approximately 2 weeks resulting in some changes in water level between flights as discharge fluctuated in the

river. These misalignments between the derived water surface points were edited out of the final dataset in Caris HIPS as much as possible, but some fluctuations remain.

Product Development

LAS Point Cloud

A tiled LAS point cloud was developed for the project area covering the topo-bathymetric lidar extents. Tiles are 500m x 500m in size and are clipped to the defined project area. The LAS files are named by the bottom left-hand corner using the first four digits of the easting value followed by digits 2 through 5 of the northing value. The integration and product development workflow is shown in Table 10. Detailed classification descriptions are provided in Appendix B.

Data Integration Step	Description			
Convert and classify data	Convert ASCII sonar and conventional survey data to *.LAS v1.4 PDRF 8. Add classifications for sonar and topo-bathymetric lidar.			
Terrain Development and Inspection	Develop terrain using appropriate point classifications. Add additional breaklines to sonar and lidar datasets to force correct interpolation of features. Visual inspect terrain for correct interpolation.			
Data Accuracy Assessment	Query the terrain dataset with lidar, Conventional and Sonar QAQC GSCPs. Develop statistics based on standards set in place by the American Society for Photogrammetry and Remote Sensing Edition 2, Version 1 (ASPRS, 2023).			
Final Products	Develop and review final products requested by client.			

Table 10: Data Integration Workflow

Terrain Model

A raster digital terrain model at 1-m resolution was developed for the project area. Ground (2), Bathymetric Lidar High Confidence (40), Bathymetric Low Confidence (66) and MBES Binned (81) classes were used as the basis for the terrain. Breaklines were developed as needed to enforce correct terrain interpolation. Additional water's edge breaklines were developed around ponds, lakes, and side channels and were used to hydroflatten the terrain model.

Data Voids

The water's edge breaklines delineate disconnected waters such as ponds, lakes and side channels. These areas contain sparse or no bathymetric data. Where bathymetric data was sparse and unable to produce and continuous surface, these polylines were used as breaklines to hydro flatten the terrain model.

Ortho Imagery

The orthorectified imagery was produced at 10 cm resolution and was tiled into 500m x 500m tiles. The tiles are delivered as tiff files. Additionally, the imagery was mosaiced and exported as both JPEG 2000 and tiff files. The mosaiced imagery is split into five segments, designated by the closest river mile, as shown in Figure 8.

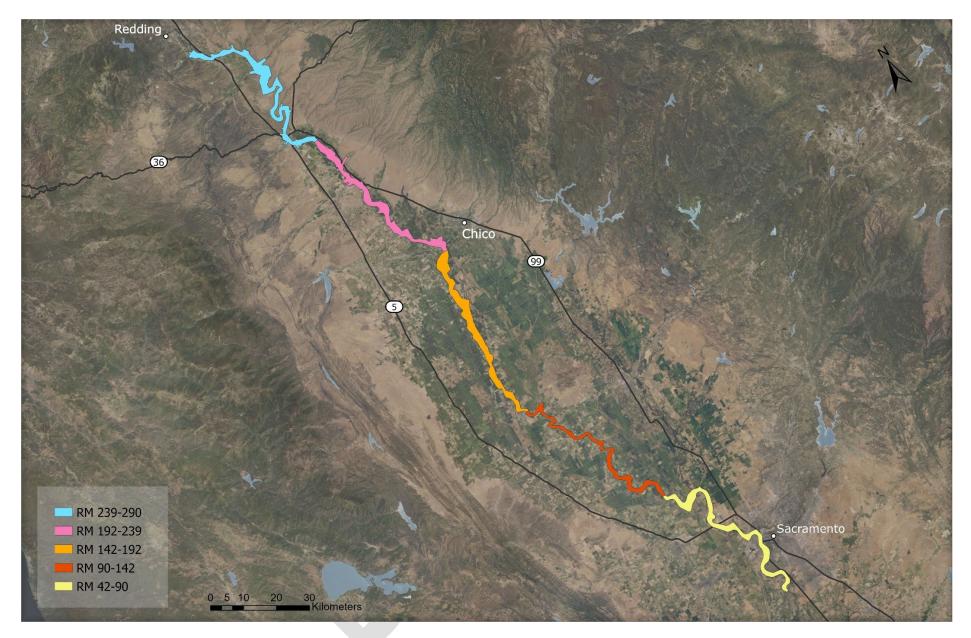


Figure 8: Orthoimagery Segments

Deliverables

The deliverables generated for the Sacramento River project are shown in Table 11. The deliverables were provided electronically in an external hard drive, accompanying this report.

Table 11: Survey Dat	a Deliverables		_		
		Data Deliverables			
Horizontal Datum: NAD83 (2011) Epoch 2010.0					
	Vertical Datum	: NAVD88 (GEOID18)			
	Projection: UTM	Zone 10N (EPSG:6339)			
	Uni	its: Meters			
Point Cloud	Tiled Point Cloud				
Data Package		• Classified Point Cloud (*.las)			
	Tiled Terrain Model				
Terrain Surface Data Package		• 1-m DEM (*.tif)			
Data I ackage		• 1-m Hillshade (*.tif)			
	Imagery Mosaics				
Imagery Data		• 4-Band orthorectified image mosaics (*.jp2)			
Package	Tiled Imagery				
• 4-Band orthorectified imagery (*.tif)					
		• Topobathy-Lidar Extents (.shp)			
		• Imagery Extents (*.shp)			
Vector Data Package		• Tile Index (*.shp)			
V CUUI Data I ackage		• Breaklines (*.shp)			
		• Data Voids (*.shp)			
		• Water Surface Profiles (*.shp)			
	Ground Survey Points				
Accuracy		• Aerial Targets and Vertical Accuracy Points			
Assessment	A	(*.csv)			
Data Package	Accuracy Reports				
		• Statistical Analysis Summary (*.txt)			

Table 11. Survey Data Deliverables

ACCURACY ASSESSMENT

Horizontal and vertical accuracy assessments of topo-bathymetric lidar and imagery products follow the guidelines set forth in the American Society for Photogrammetry and Remote Sensing Edition 2, Version 1 (ASPRS, 2023). Absolute accuracy is evaluated using the Root Mean Square Error (RMSE) values for the digital data and the mean reported ground survey accuracy values.

Absolute Vertical Accuracy

Absolute vertical accuracy was assessed using Non-Vegetated Vertical Accuracy (NVA) reporting. NVA accuracy reporting compares point data collected in areas with open sky, where a lack of vegetation exists and on surfaces with level slopes ($<10^{\circ}$) to the triangulated irregular

network (TIN) surface generated by the survey data. The accuracy test was performed using 159 check points. The vertical accuracy of the topo-bathymetric lidar data expressed as the Vertical Product Root Mean Square Error (RMSEv) is 0.063 m and was calculated using the following ASPRS equation (ASPRS, 2023):

$$RMSE_V = \sqrt{RMSE_{V1}^2 + RMSE_{V2}^2}$$

Where:

 $RMSE_{V1}$ = RMSE_Z of the tested product.

 $RMSE_{V2}$ = The mean vertical accuracy of the ground surveys, reported by the field surveyor.

The results of the absolute accuracy assessment are shown in Table 12. The full accuracy assessment report including the minimum and maximum error, the standard deviation, and the residual errors at each check point can be found in the Accuracy Data Package.

 Table 12: Non-Vegetated Vertical Accuracy Assessment (NVA)

Non-Vegetated Vertical Accuracy (NVA)				
Samples 159				
Mean (m)	0.031			
$RMSE_{v1}(m)$	0.047			
$RMSE_{v2}(m)$	0.026			
RMSE _v (m)	0.054			

Orthoimagery Accuracy

Image accuracy was assessed using air targets that were withheld from initial aerial triangulation. These checkpoints were identified in the adjusted orthoimagery and the displacement from targets centers was recorded for statistical analysis. The horizontal accuracy was assessed using independent, evenly distributed air targets. The horizontal accuracy test was performed using ninety-three (93) check points. The horizontal accuracy of the imagery expressed as the Planimetric Root Mean Square Error (RMSE_H) is 0.089 m, and was calculated using the following ASPRS equation (ASPRS, 2023):

$$RMSE_{H} = \sqrt{RMSE_{H1}^{2} + RMSE_{H2}^{2}}$$

Where:

 $RMSE_{H1} = RMSE_{XY}$

 $RMSE_{H2}$ = The mean accuracy of the ground surveys, reported by the field surveyor.

The results of the horizontal accuracy assessment are shown in Table 13. The full accuracy assessment report including the minimum and maximum error, the standard deviation, and the residual errors at each check point can be found in the Accuracy Data Package.

Orthoimagery Check Point Accuracy Assessment							
	Air Targetsx Air Targetsy Air Targetsxy						
Samples	93						
Mean (m)	0.033	0.021	0.074				
RMSE _{H1} (m)	0.065	0.058	0.087				
RMSE _{H2} (m)							
RMSE _H (m)			0.089				

Table 13: Orthoimagery Horizontal Accuracy Assessment

REFERENCES

ASPRS Positional Accuracy Standards for Digital Geospatial Data (Edition 2, Version 1.0. – August 23, 2023).

https://publicdocuments.asprs.org/PositionalAccuracyStd-Ed2-V1

APPENDICES

Appendix A Ground Survey Position Checks

Nterrer	DID	Difference		Communit.	
Name	PID	dN	dE	dH	Comment
HPGN D CA 03 BG	AC9219	-0.010	0.001	0.100	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.002	-0.002	0.070	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.016	0.001	0.060	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.013	0.004	0.066	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.014	0.011	0.061	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.023	0.005	0.056	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.004	0.004	0.069	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.016	0.005	0.046	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.009	0.003	0.040	Check in from WD1J
HPGN D CA 03 BG	AC9219	-0.007	-0.005	0.440	Check in from WD1J
F 859 RESET	AI5062	-0.025	-0.007	0.387	Check in from WD1J
F 859 RESET	AI5062	-0.023	-0.006	0.410	Check in from WD1J
RIVER	AI5069	-0.008	-0.010	0.066	Check in from WD1J
RIVER	AI5069	-0.004	-0.002	0.065	Check in from WD1J
RIVER	AI5069	-0.005	-0.007	0.055	Check in from WD1J
RIVER	AI5069	0.003	-0.018	-0.007	Check in from WD1J
RIVER	AI5069	-0.002	-0.019	0.021	Check in from WD1J
TYNDALL	AI5072	-0.039	-0.011	0.306	Check in from WD1J
TYNDALL	AI5072	-0.041	-0.016	0.320	Check in from WD1J
5 SHA 11.70	DH6394	0.031	-0.013	-0.003	Check in from RD1L
5 SHA 11.70	DH6394	0.029	-0.014	-0.007	Check in from RD1L
5 SHA 11.70	DH6394	0.030	-0.016	-0.006	Check in from RD1L
5 SHA 11.70	DH6394	0.023	-0.012	-0.001	Check in from RD1L
5 SHA 11.70	DH6394	0.020	-0.018	0.003	Check in from RD1L
WILKENS	DH6502	-0.011	0.019	0.054	Check in from YC1I
WILKENS	DH6502	-0.009	0.022	0.073	Check in from YC1I

Norma	DID	PID			Comment
Name	PID	dN	dE	dH	Comment
WILKENS	DH6502	-0.011	0.019	0.054	Check in from YC1I
WILKENS	DH6502	-0.009	0.022	0.073	Check in from YC1I
MICHIGAN	DH6625	0.013	0.022	0.030	Check in from CH1G
MICHIGAN	DH6625	0.009	0.009	-0.003	Check in from OR1K
BEND BRIDGE	DL9142	-0.004	0.008	-0.037	Check in from RD1L
BEND BRIDGE	DL9142	0.004	0.013	-0.061	Check in from RD1L
BEND BRIDGE	DL9142	0.022	-0.006	0.004	Check in from RD1L
BEND BRIDGE	DL9142	0.010	-0.001	-0.008	Check in from RD1L
ORDBEND	DL9190	0.005	-0.001	0.055	Check in from CH1G
ORDBEND	DL9190	0.010	0.005	0.023	Check in from CH1G
PELGER	DL9193	-0.014	0.027	0.050	Check in from YC1I
PELGER	DL9193	-0.010	0.024	0.038	Check in from YC1I
PELGER	DL9193	-0.014	0.018	0.068	Check in from YC1I
PELGER	DL9193	-0.014	0.019	0.062	Check in from YC1I
EGOC	DN4101	-0.003	0.010	0.102	Check in from SACR
EGOC	DN4101	-0.003	0.006	0.094	Check in from SACR
HPGN CA 03 04	KS2014	0.012	-0.016	0.098	Check in from WI1H
HPGN CA 03 04	KS2014	0.027	-0.015	0.101	Check in from WI1H
HAMILTON	KT1807	0.014	-0.017	0.017	Check in from CH1G
HAMILTON	KT1807	0.017	-0.014	0.023	Check in from CH1G
HAMILTON	KT1807	-0.003	-0.001	0.038	Check in from CH1G
HAMILTON	KT1807	0.001	-0.001	0.030	Check in from CH1G

Appendix B LiDAR Point Classification

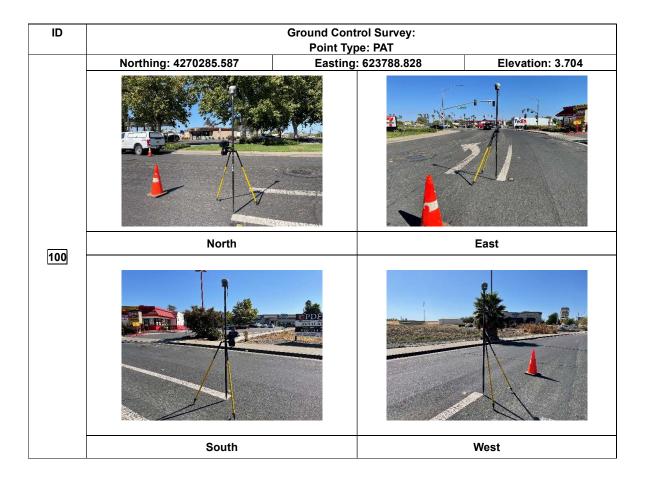
Classes shown in **bold** are used in the final terrain model.

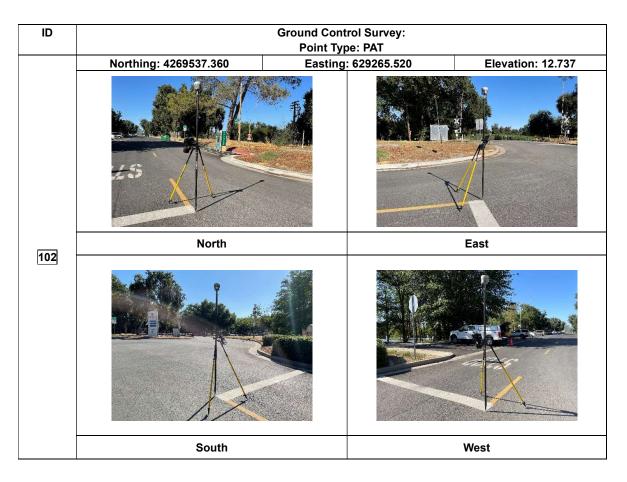
lassification Number	Classification Name	Classification Description	
1	Unclassified	Processed, unclassified topographic data	
2	Ground	Topographic laser returns that are determined to be ground using automated and manual algorithms	
7	Low Noise	Topographic lidar returns below the ground surface determined to be noise	
9	Water	Topographic laser returns that are determined to be water using automated and manual cleaning algorithms	
12	Overlap Points	Topographic laser returns where overlap between flightlines occurred	
17	Bridge Deck	Topographic laser returns that are determined to be a bridge using manual classification methods	
18	High Noise	Topographic laser returns above the ground surface determined to be noise	
20	Ignored Ground	Topographic laser ground points that were buffered along break lines	
35	Bathy LiDAR Extraneous	Manually rejected bathymetric lidar points	
40	Bathy LiDAR High Confidence	Bathymetric laser returns that are determined to be riverbed using automated and manual algorithms	
41	Bathy LiDAR Water Surface	Bathymetric laser returns determined to be water surface	
42	Bathy LiDAR Derived Water Surface	Synthetic points for bathymetric lidar refraction correction	
43	Bathy LiDAR Submerged Objects	Bathymetric laser returns determined to be submerged objects	
64	Bathy LiDAR Shallow Returns	Bathymetric laser returns determined to be noise by LSS	
65	Bathy LiDAR TWE1	Bathymetric laser returns determined to be noise by the turbid water routine in LSS	
66	Bathy LiDAR Low Confidence	Bathymetric laser returns that are determined to be riverbed using automated and manual algorithms	
67	Bathy LiDAR TWE2	Bathymetric laser returns determined to be noise by the turbid water routine in LSS	
69	Ignored Points	Manually rejected topographic and bathymetric laser returns	

Classification Number	Classification Name	Classification Description		
85	MBES Binned Ignored	Multibeam sonar points, not used in the surface.		
151	Bathy LiDAR Unclassified	Bathymetric lidar returns on land unclassified by LSS		
152	Bathy LiDAR Unclassified	Bathymetric lidar returns on land classified by LSS as ground		
157	Bathy LiDAR Low Noise	Bathymetric lidar returns below the bathymetric surface determined to be noise		
160	Bathy LiDAR High Confidence Ignored	Bathymetric lidar points rejected by manual and automated routines from high confidence		
161	Bathy Low Confidence Ignored	Bathymetric lidar points rejected by manual and automated routines from low confidence		
168	Bathy LiDAR High Noise	Bathymetric lidar returns above the bathymetric surface determined to be noise		

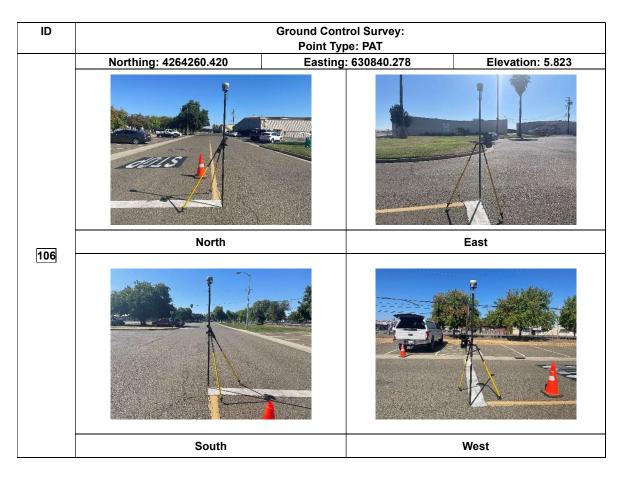
Appendix C Ground Survey Points

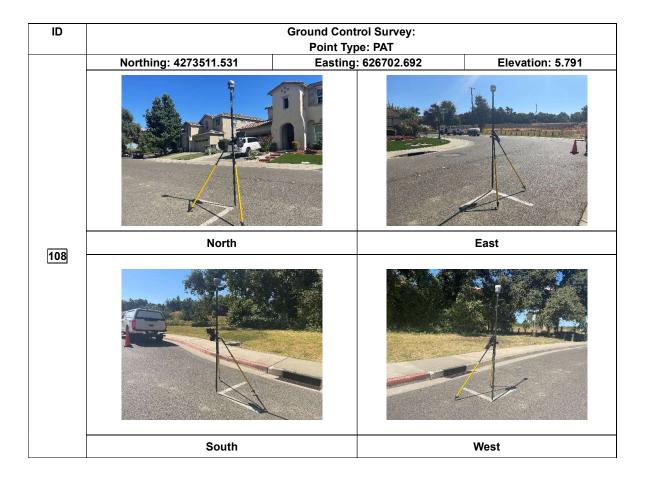
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Modified:	10/24/2023 12:46:37 (UTC:-7)	Postal Code		Units:	Meter
Time zone:	Pacific Standard Time	Country	USA	Global reference datum:	NAD83(2011)
Comment 1:		Phone		Global reference epoch:	2010
Comment 2:		Email		Geoid:	GEOID18 (Conus)
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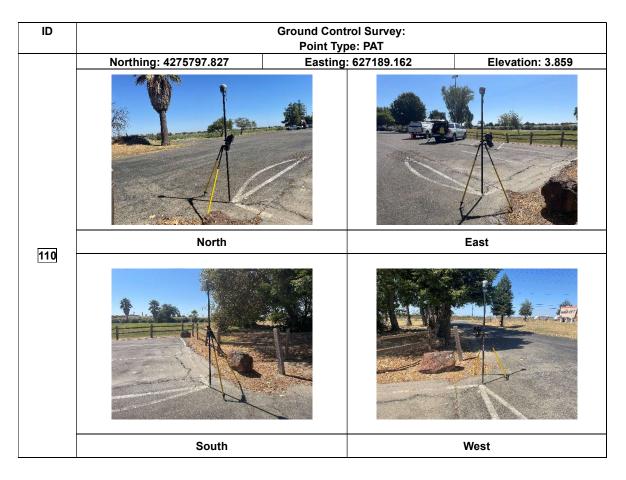


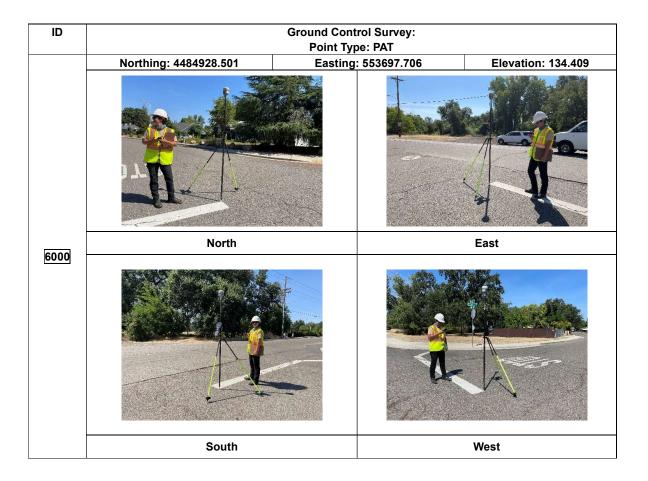


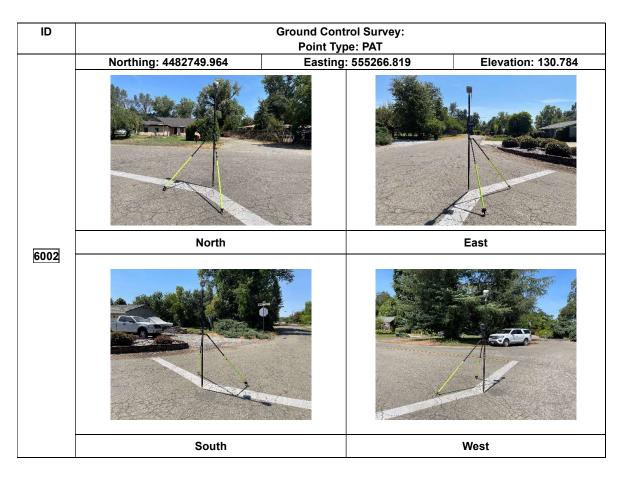


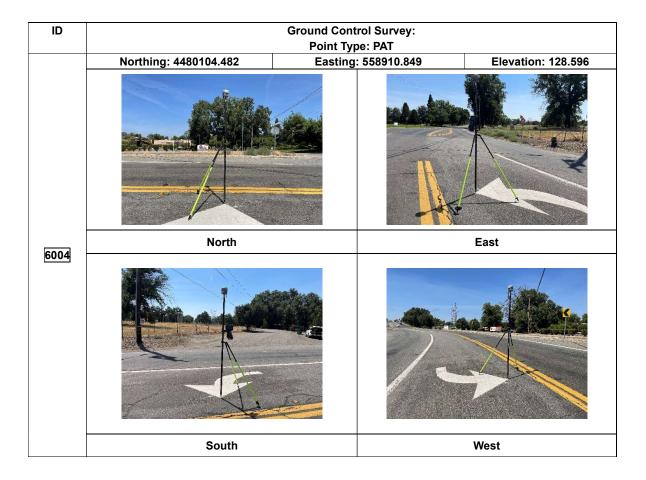




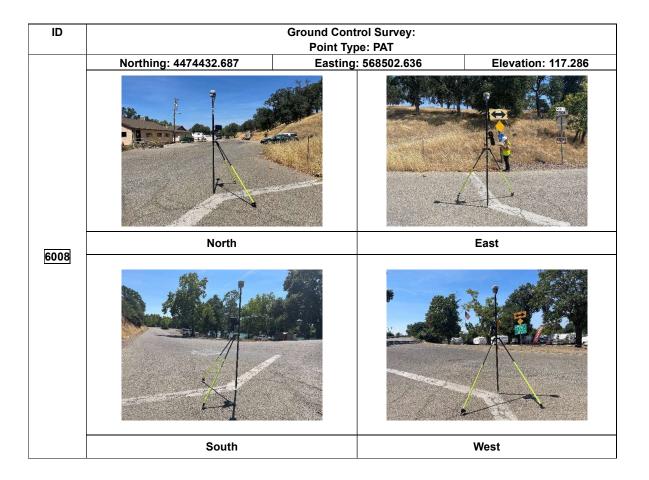






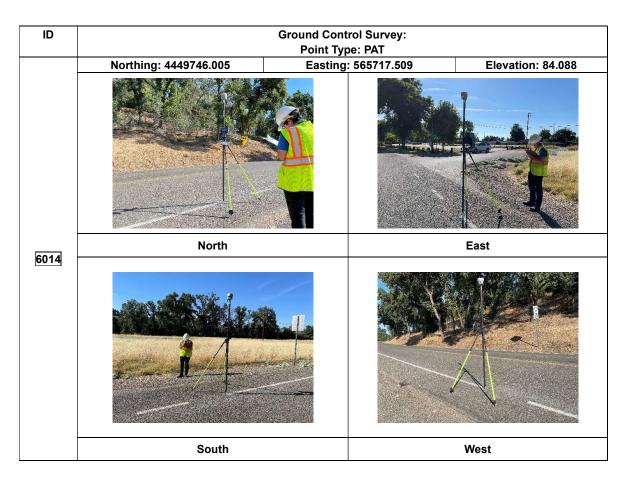


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	South		West	



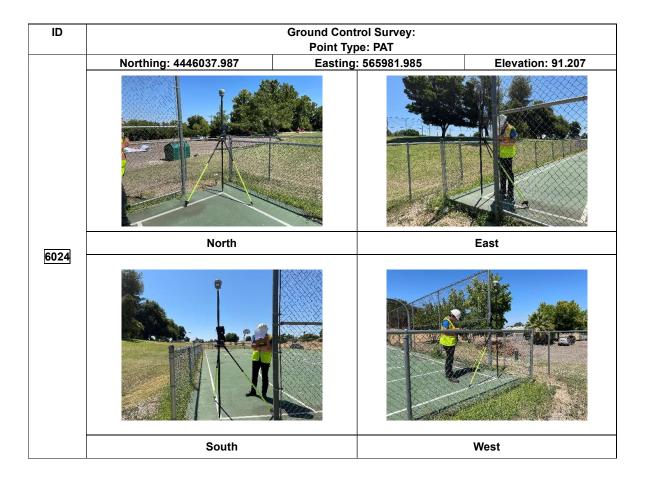


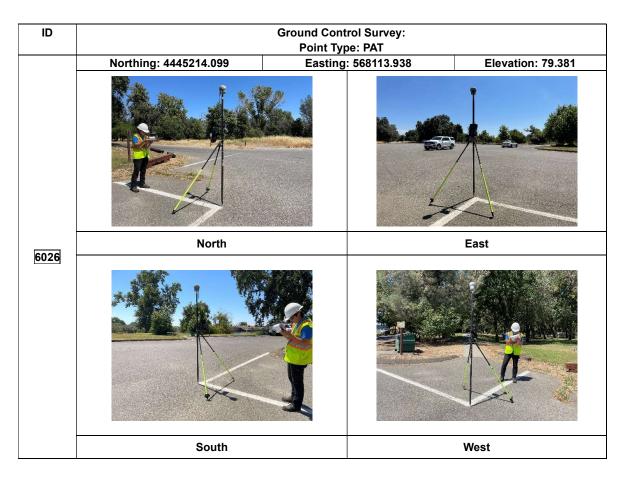


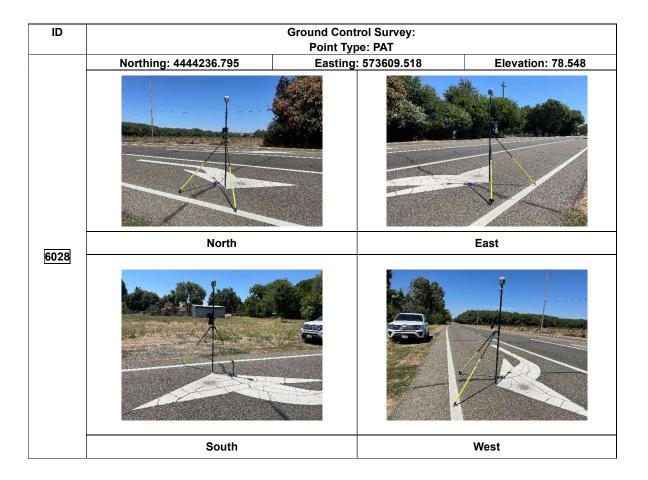


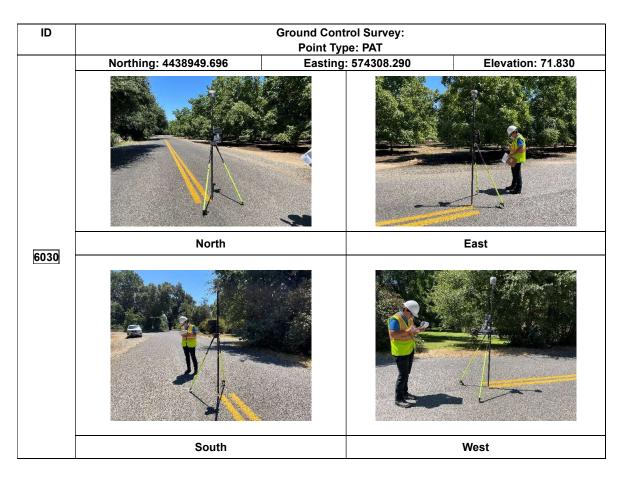
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	North		East
	South		West

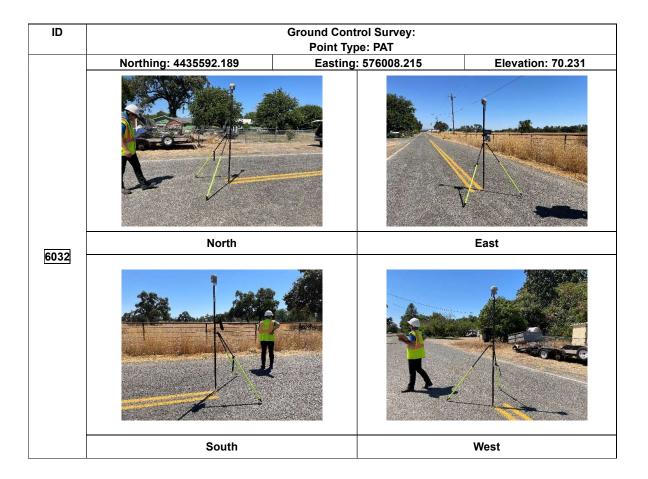


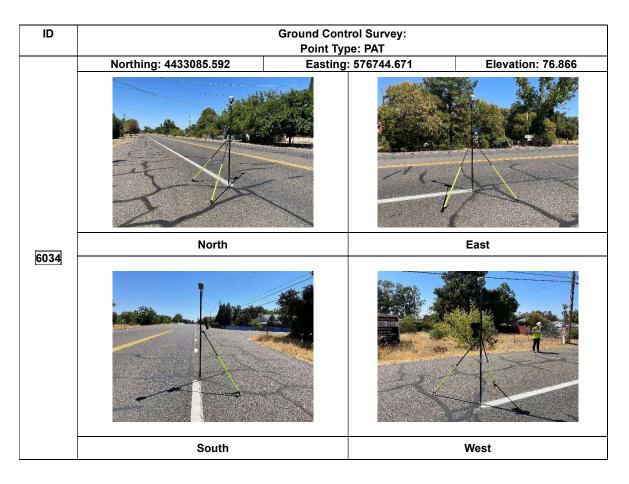




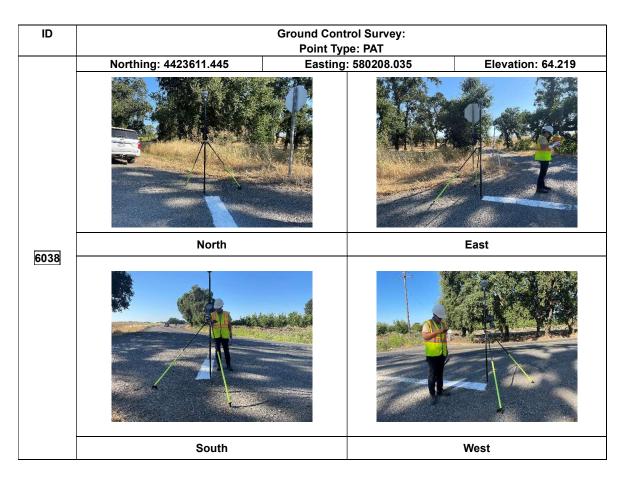


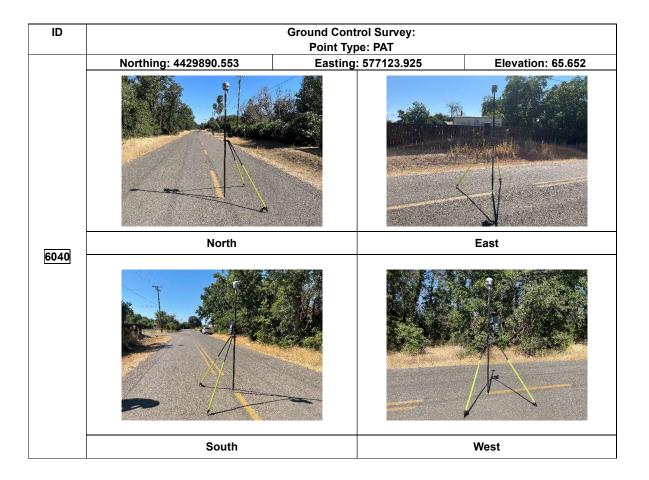


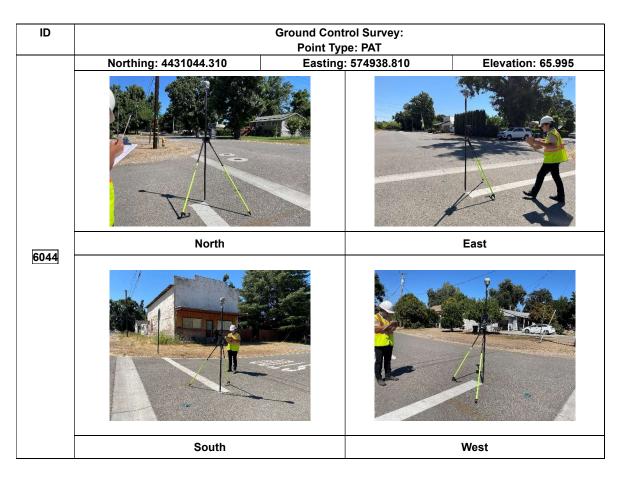


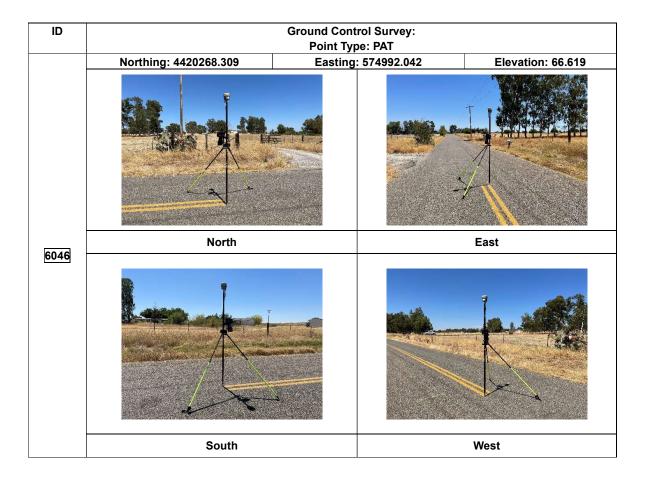








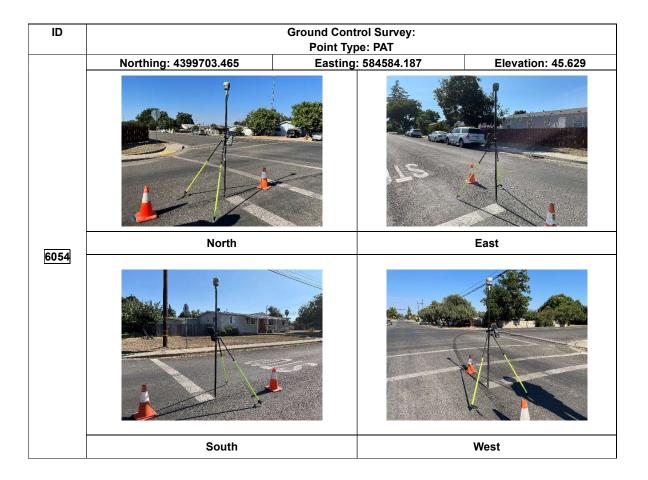


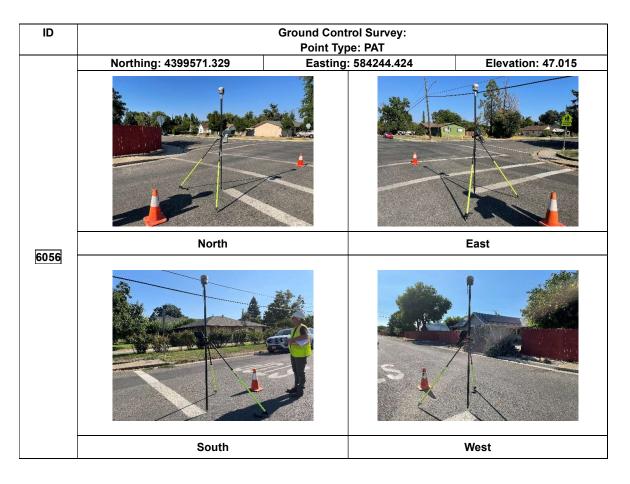


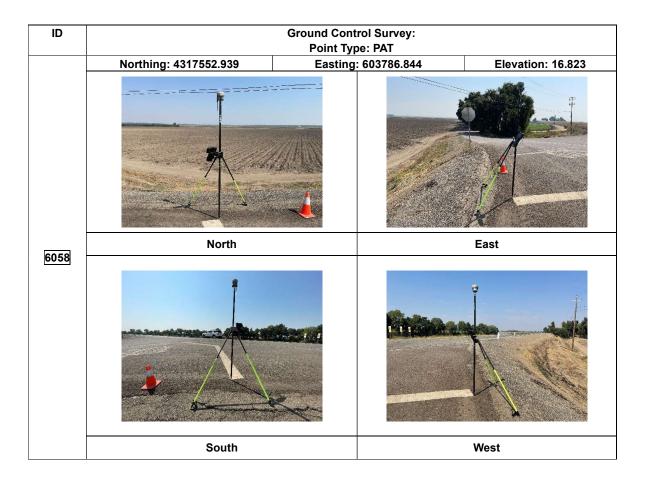


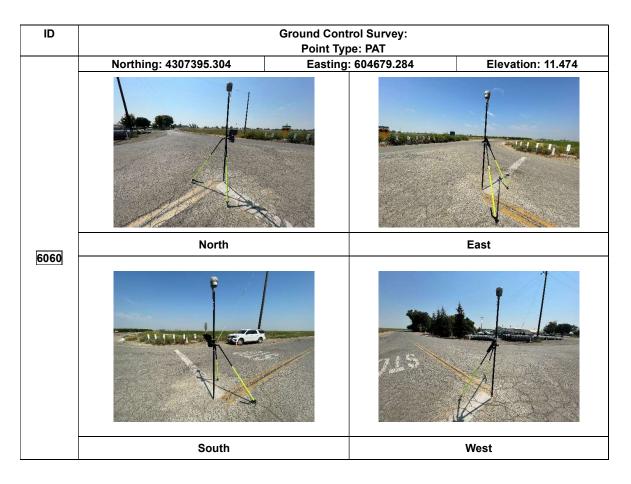
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	North		East
	South		West

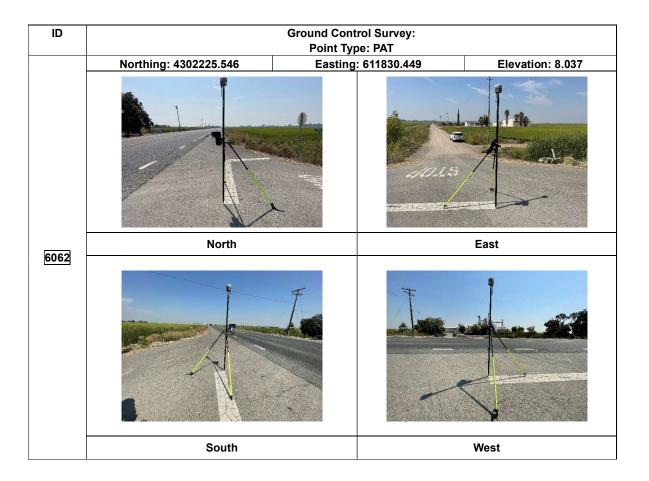


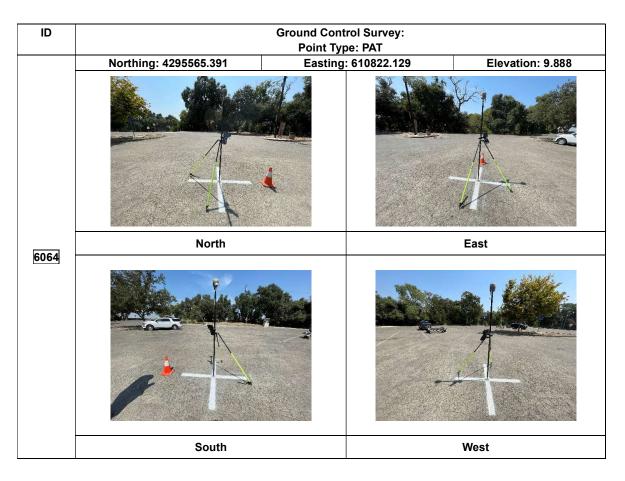


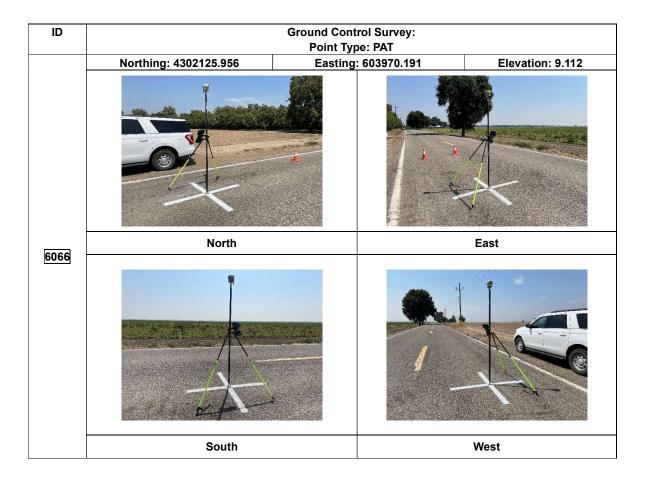


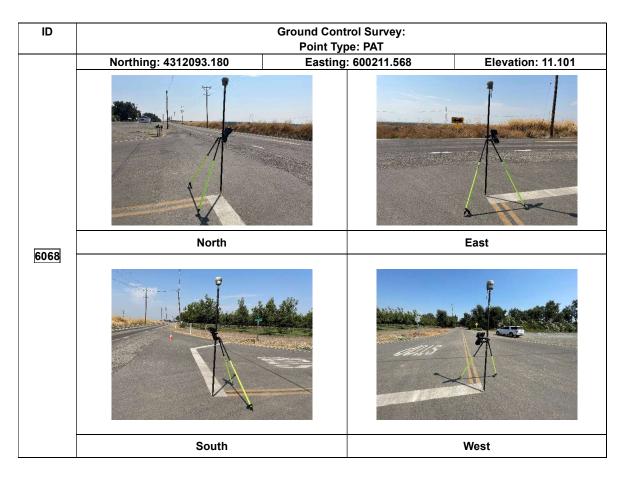


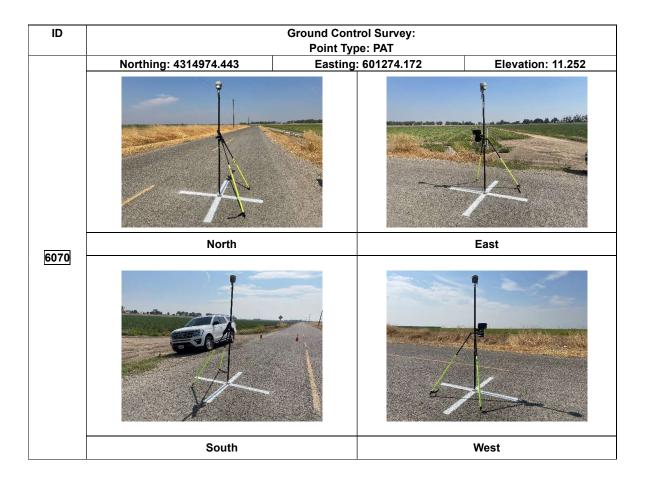


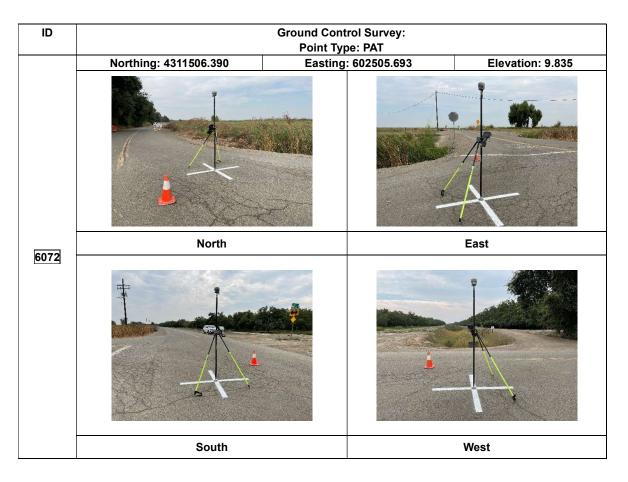


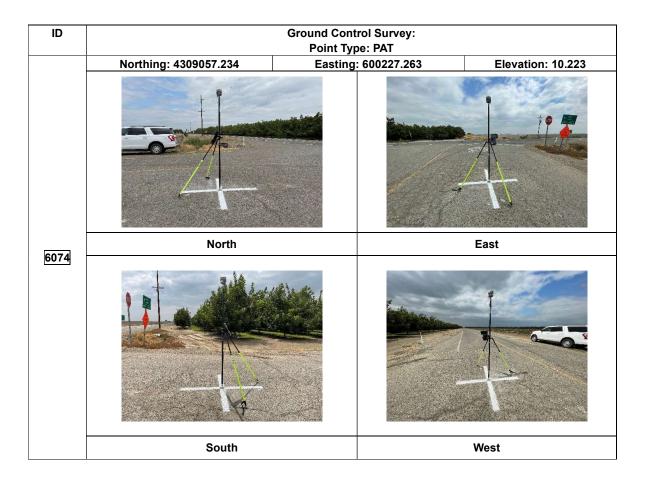


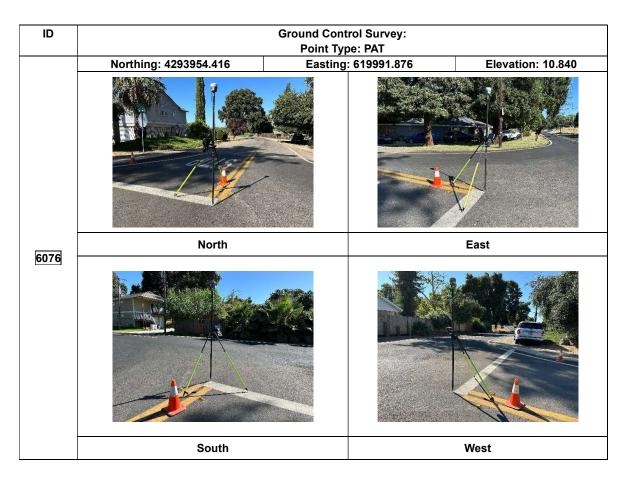


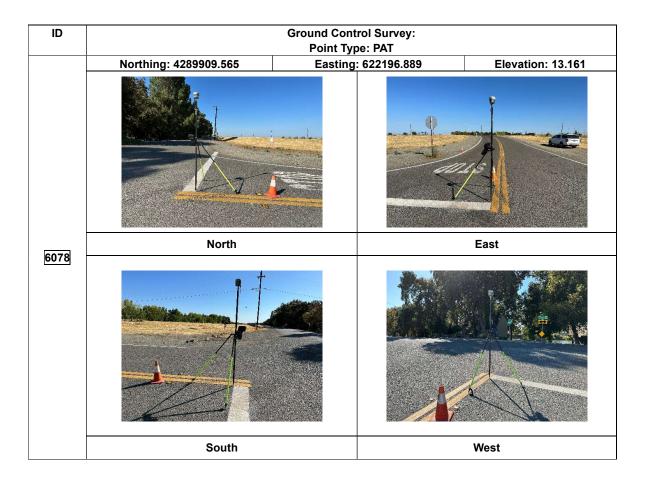


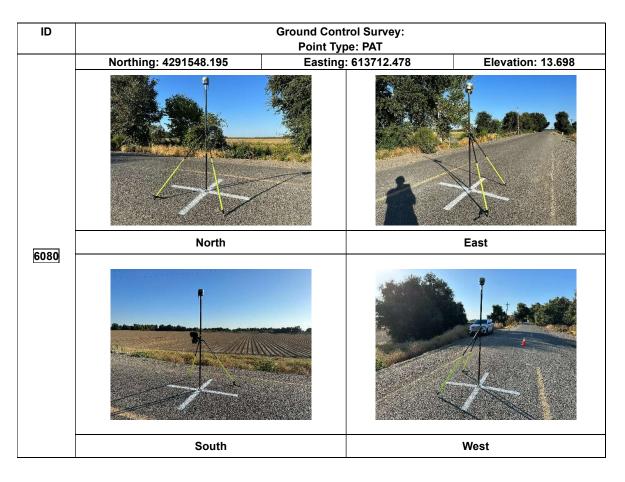


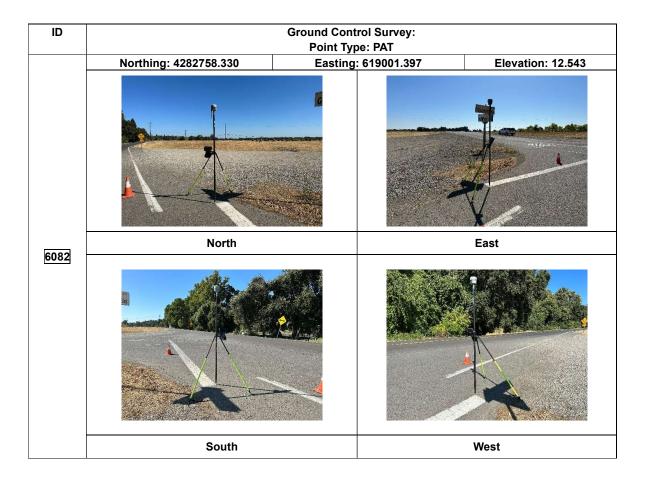


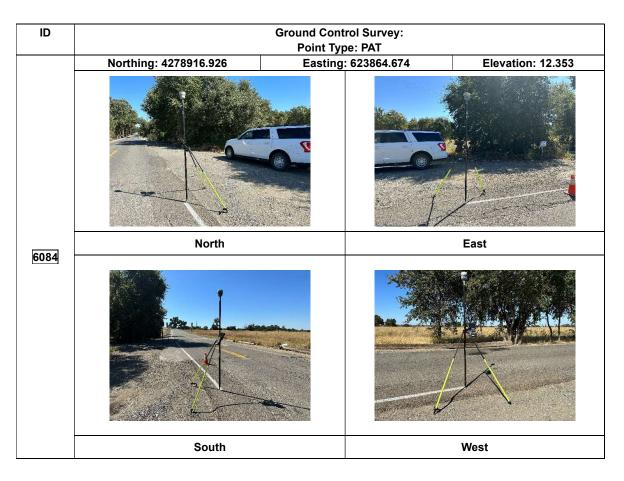


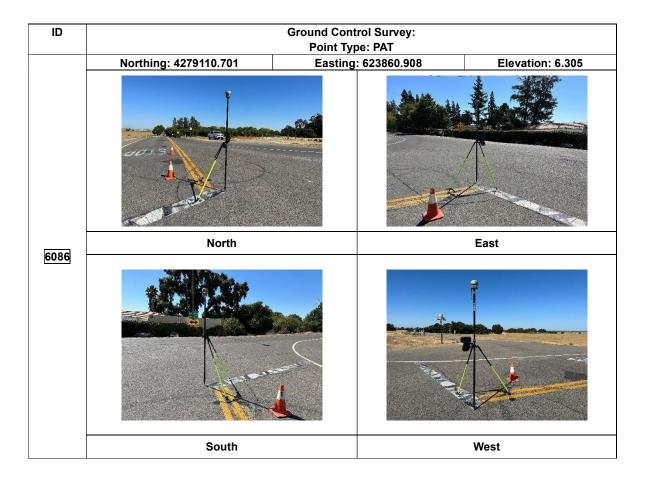


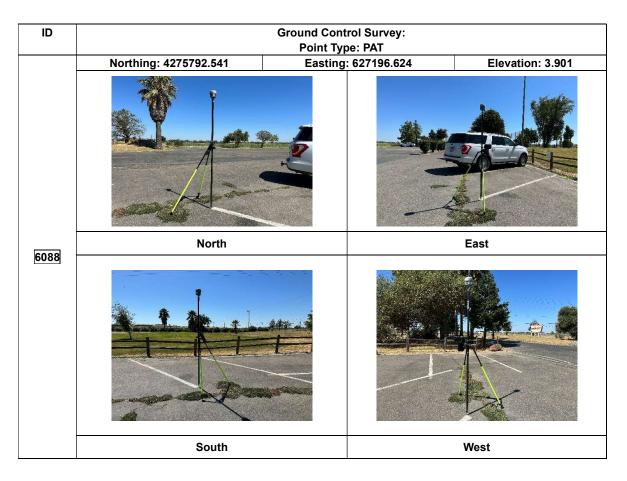


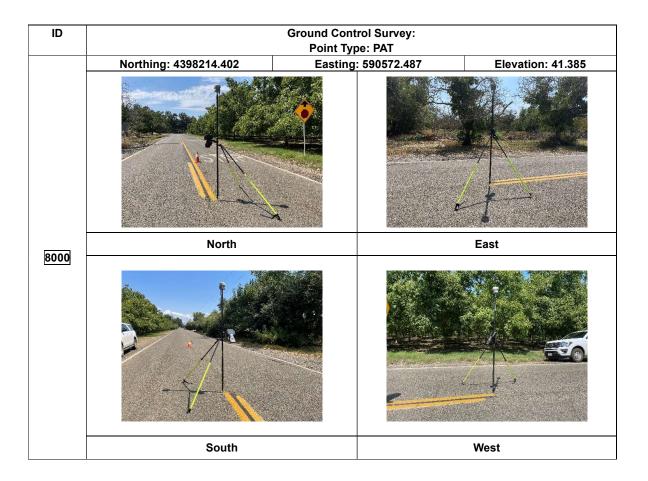


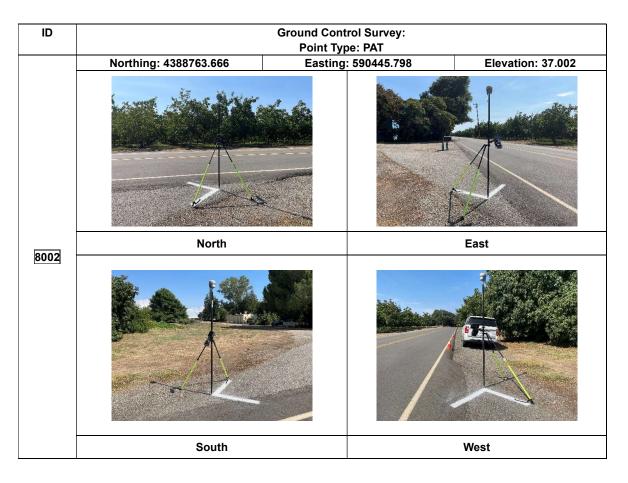


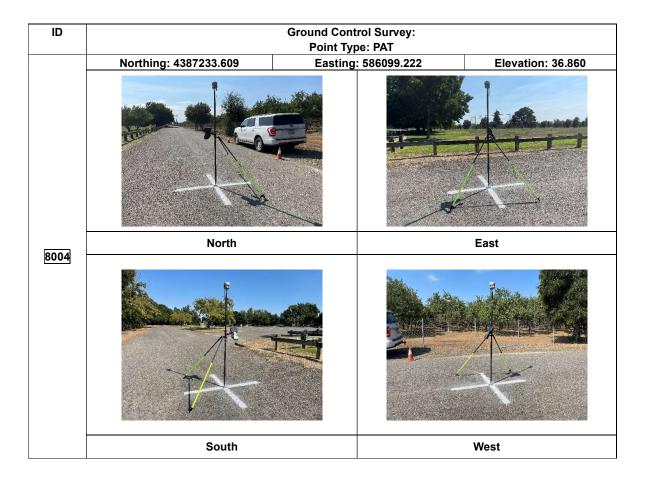


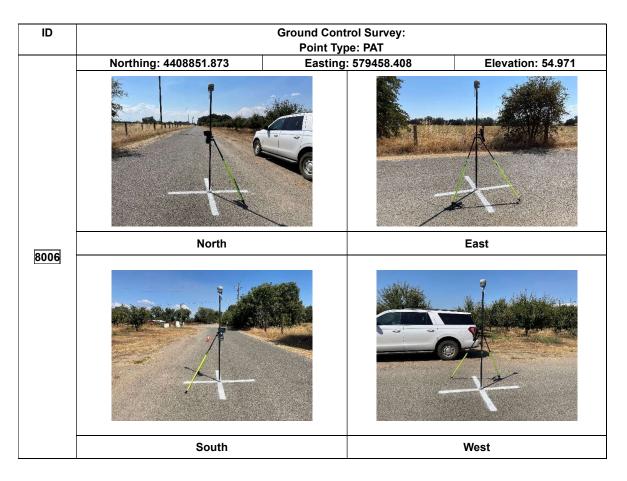




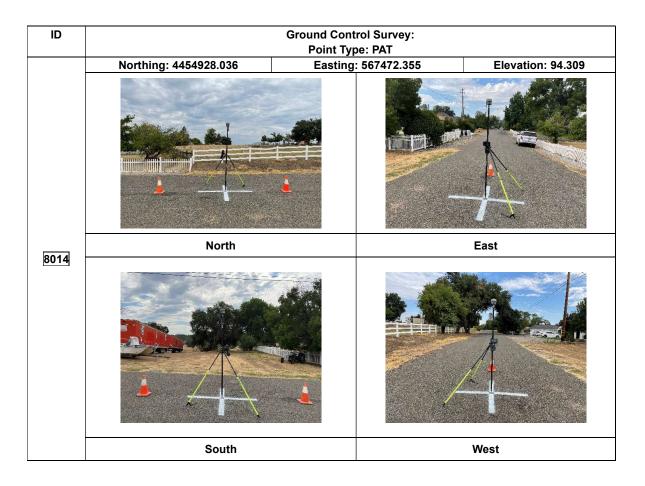


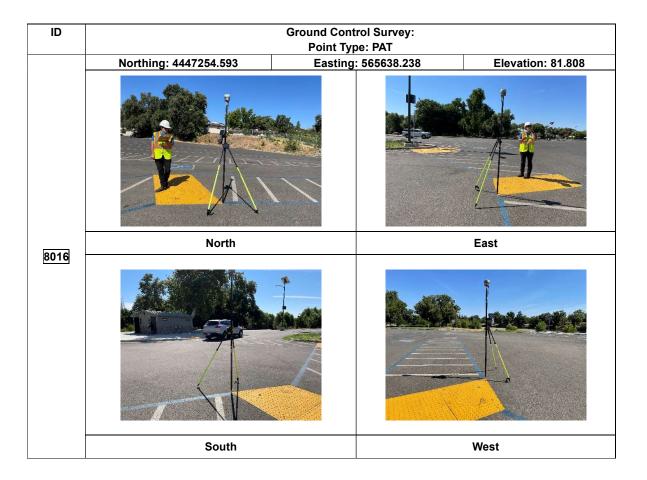


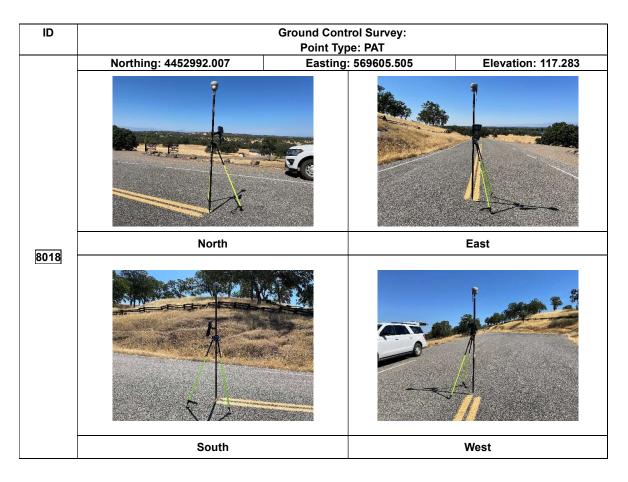


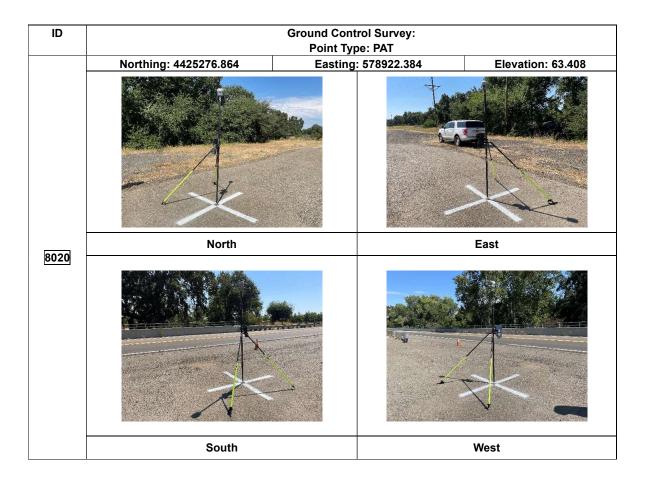


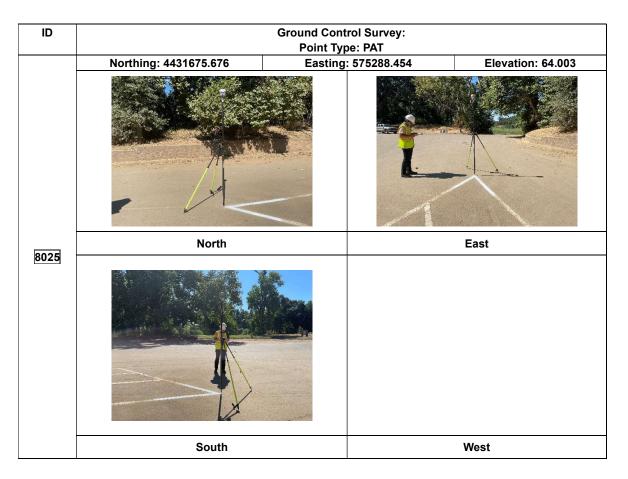
ID	Ground Control Survey: Point Type: PAT		
	Northing: 4456772.453	Easting: 566924.972	Elevation: 99.498
8012			
	North		East
	South		West

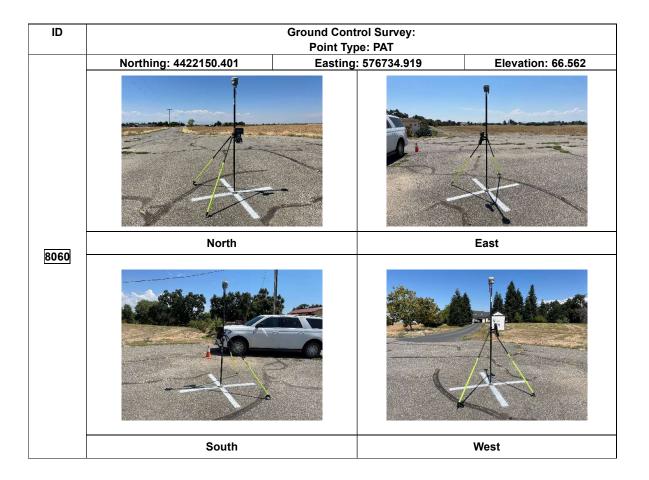


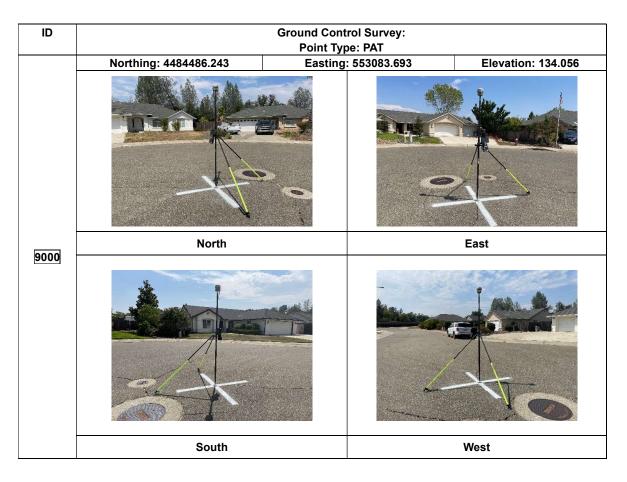


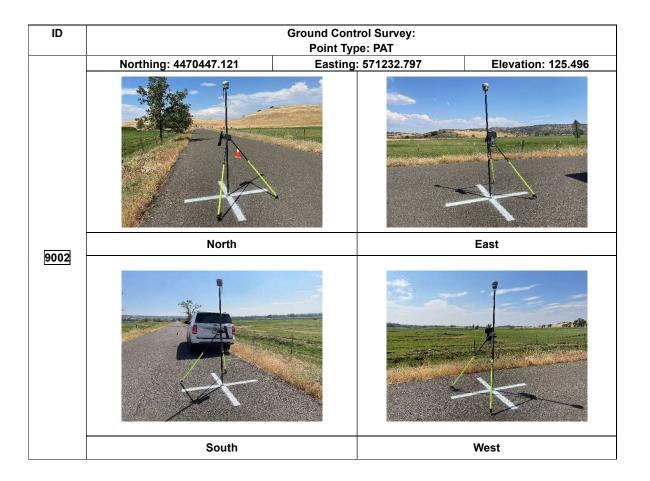


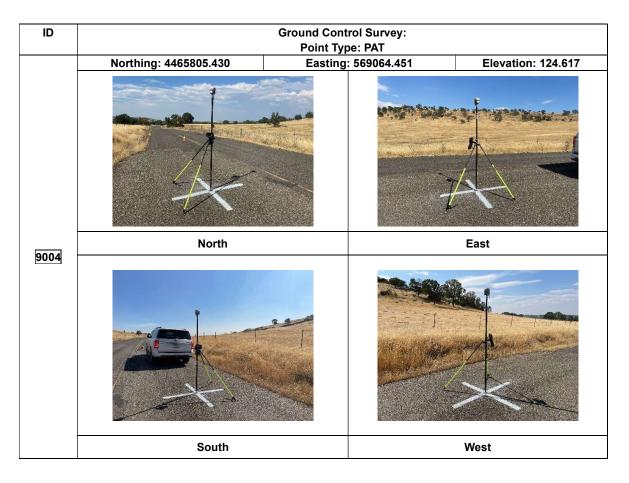


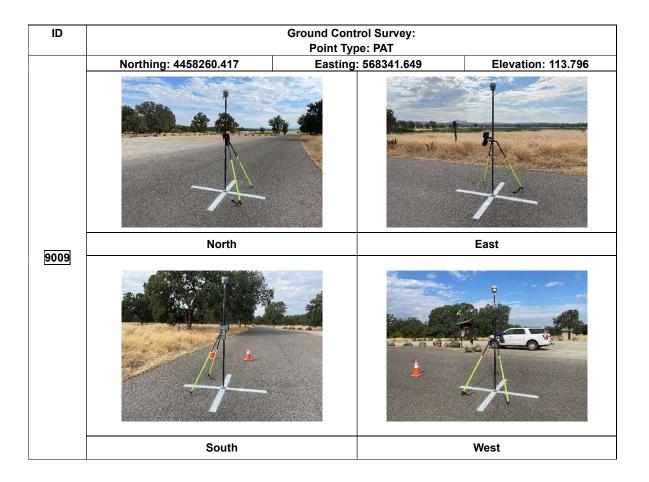


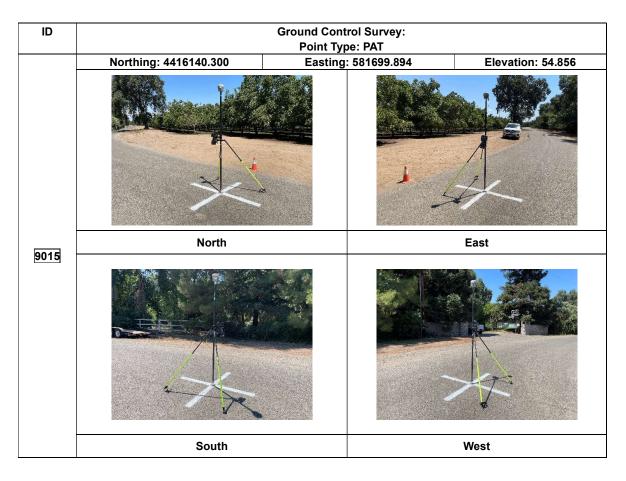


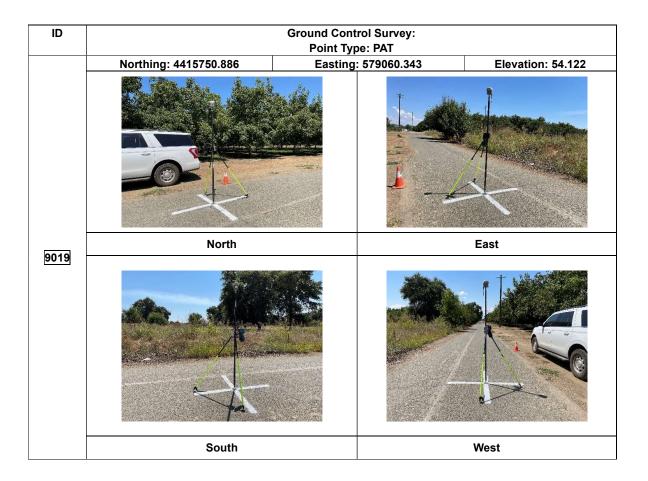


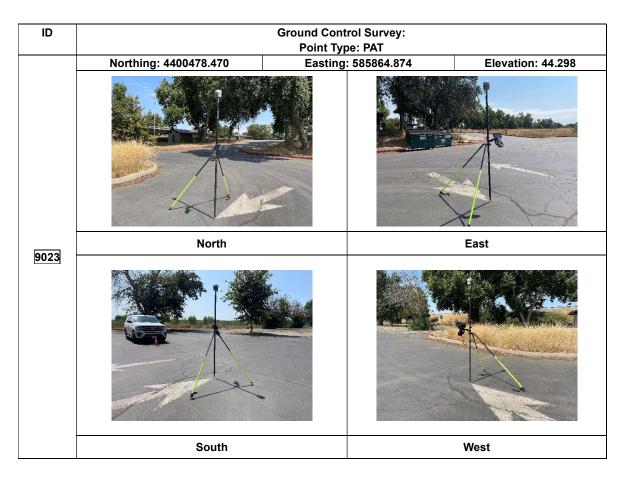


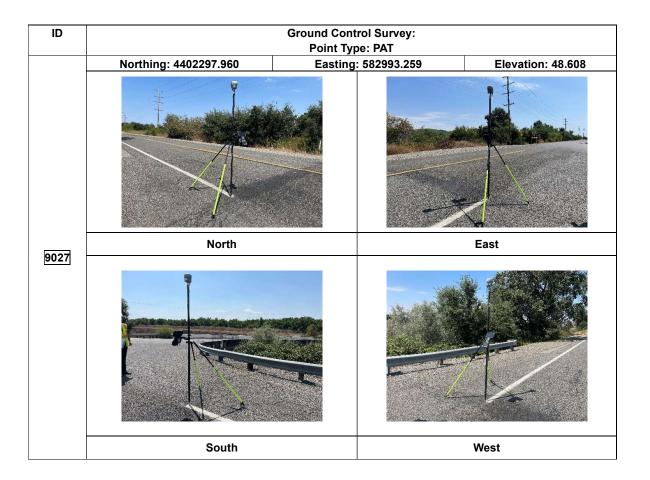


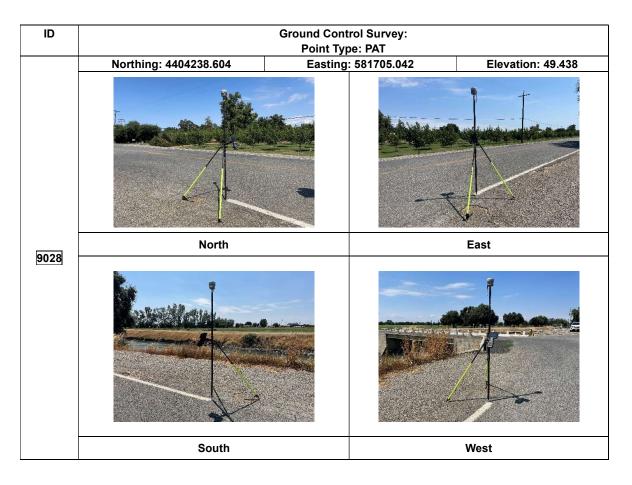


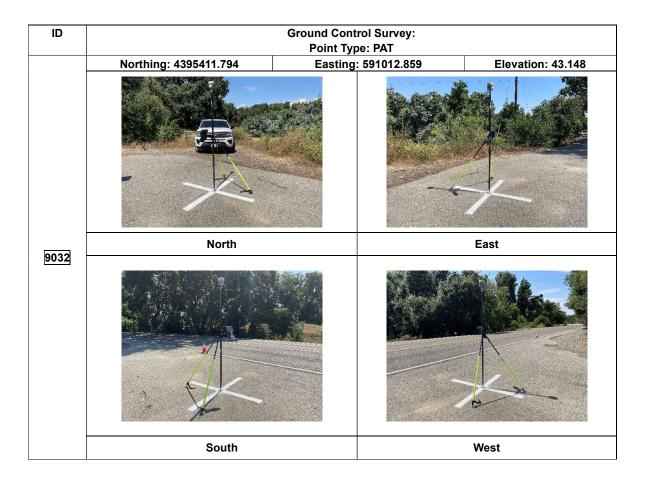


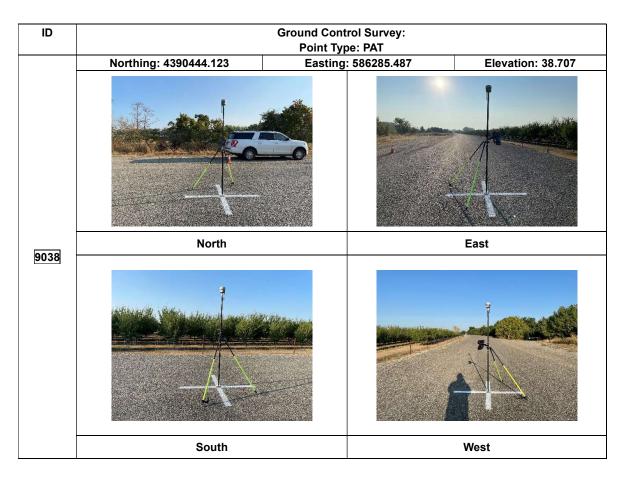


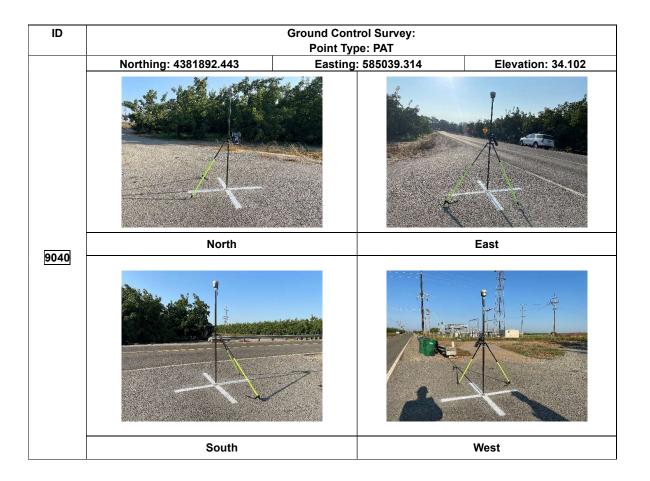


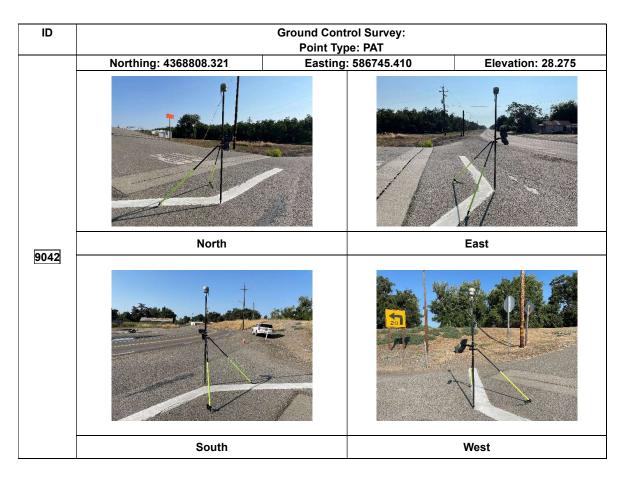


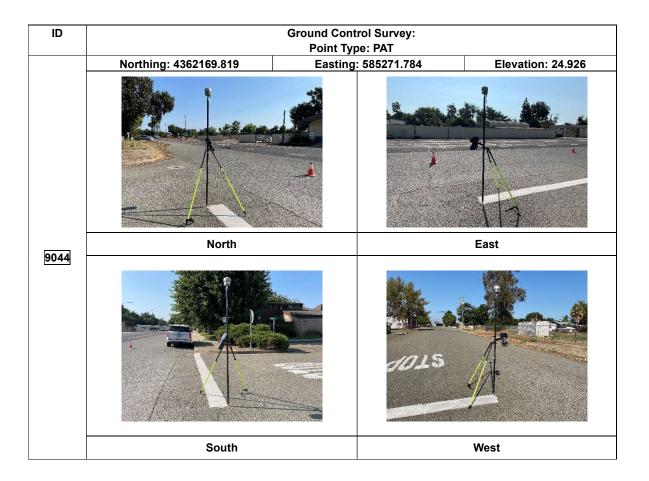


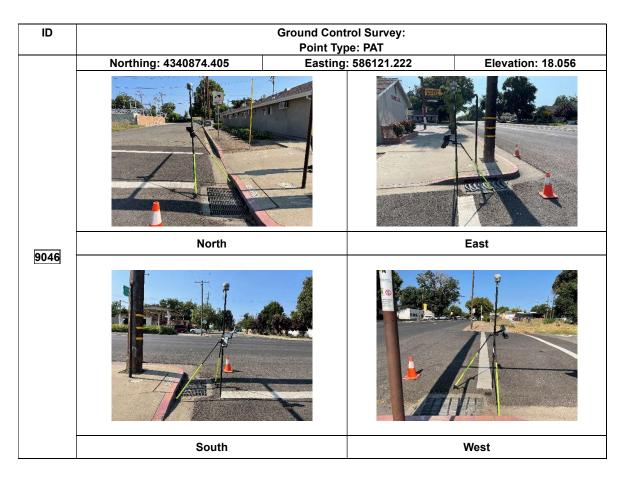


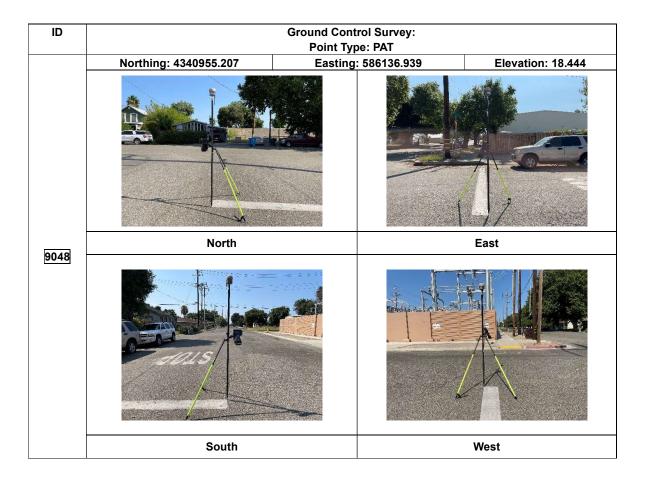


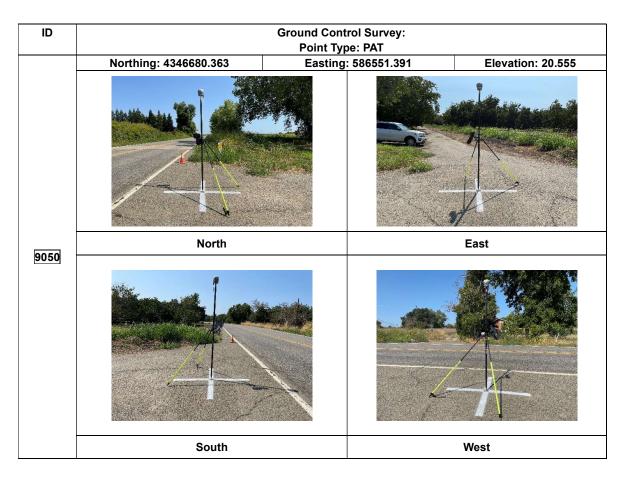


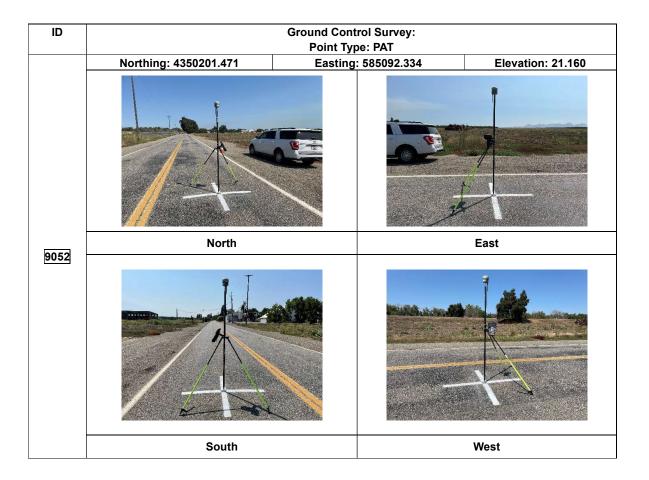


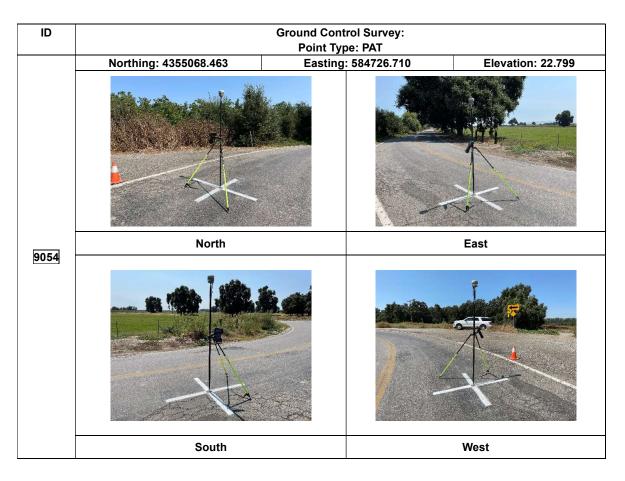


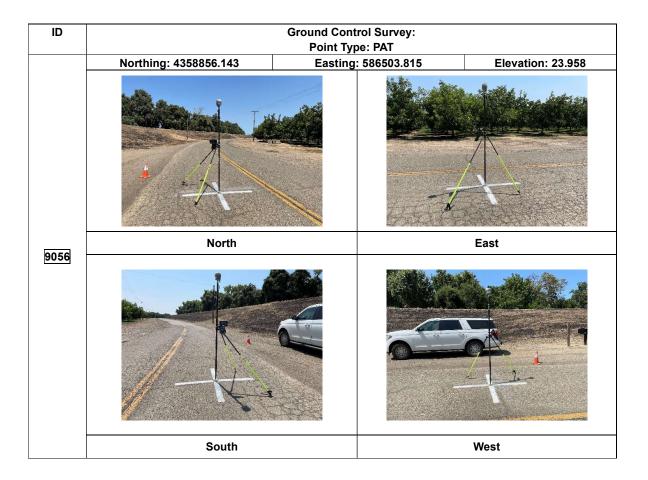


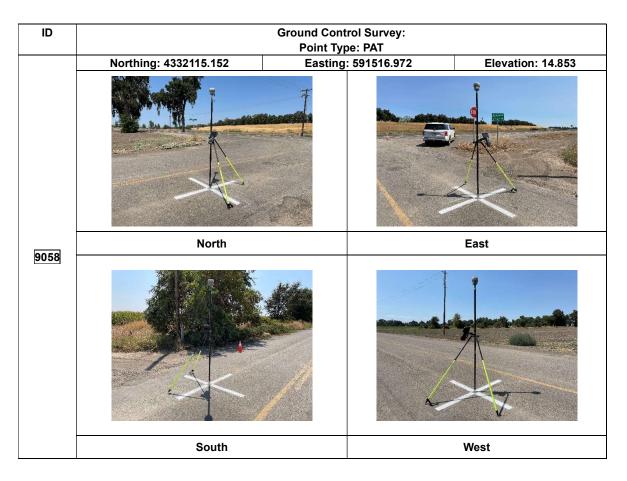


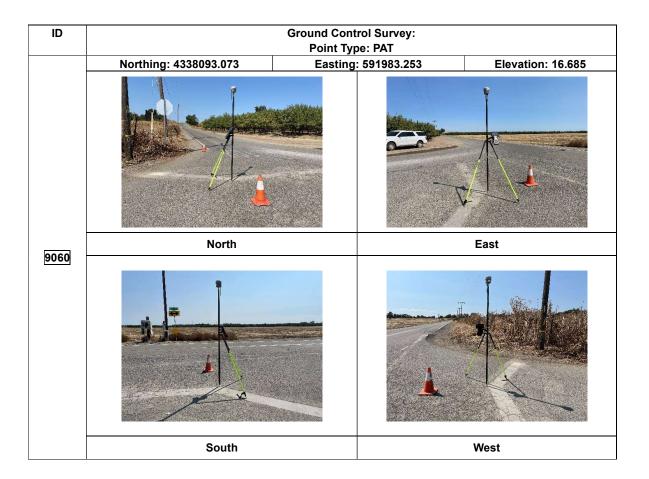


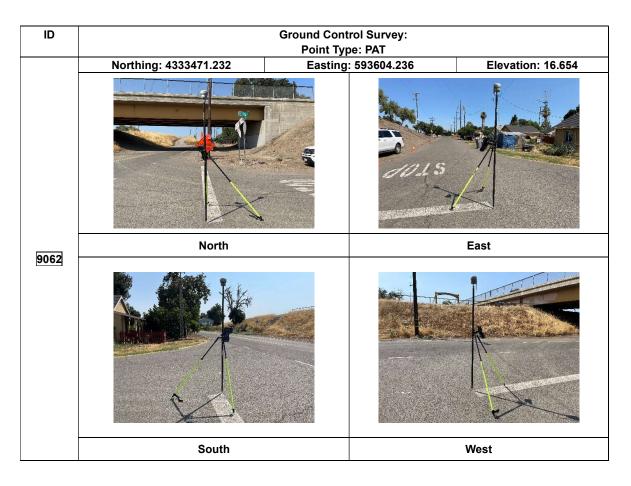


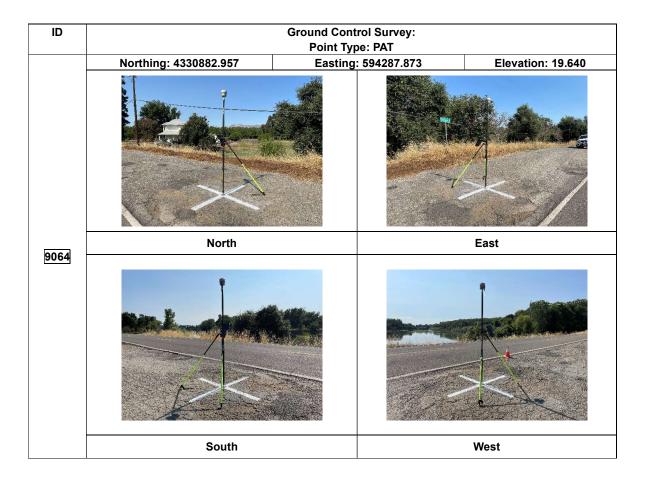


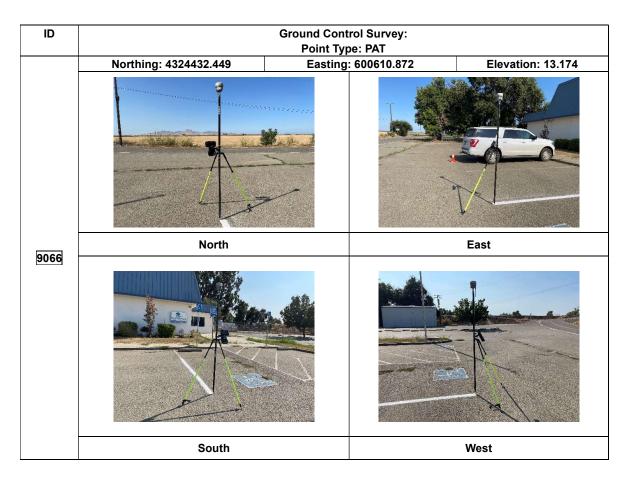


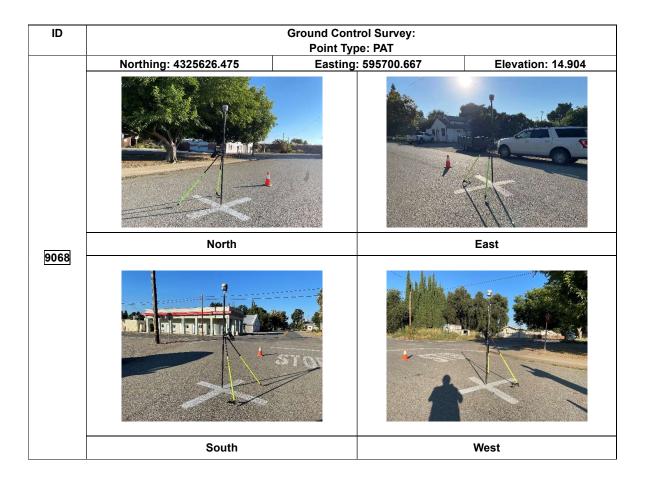


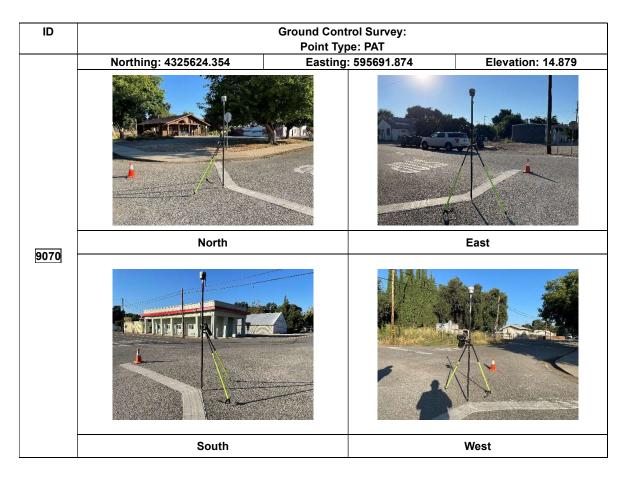


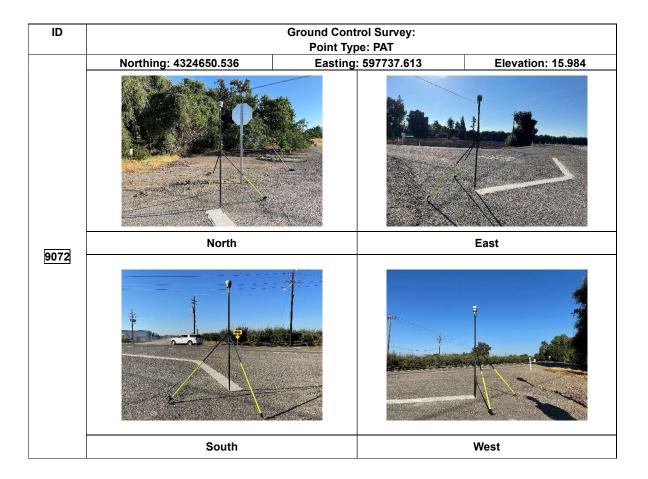


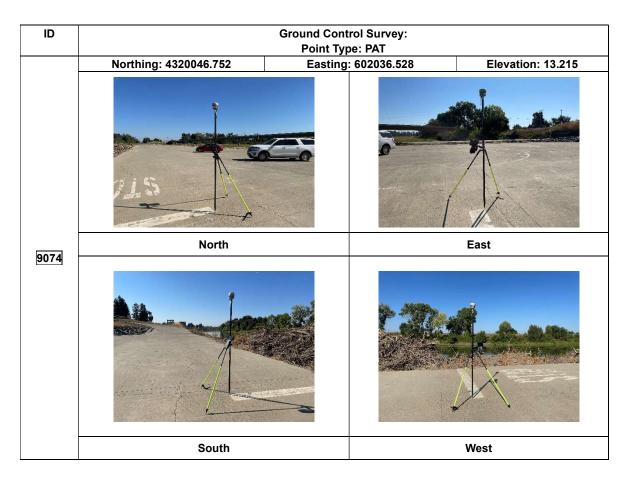


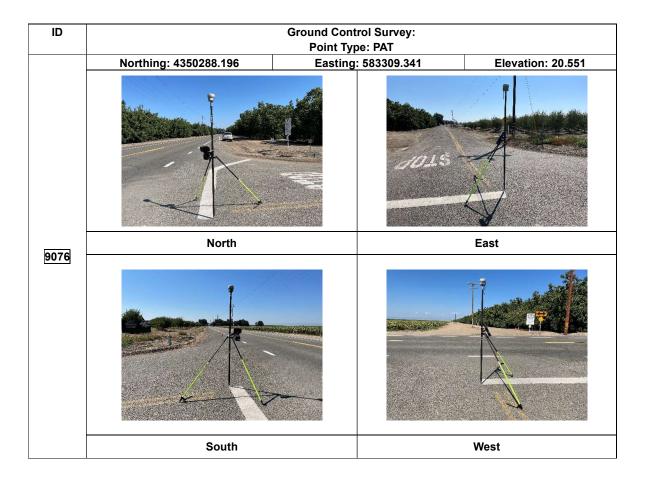


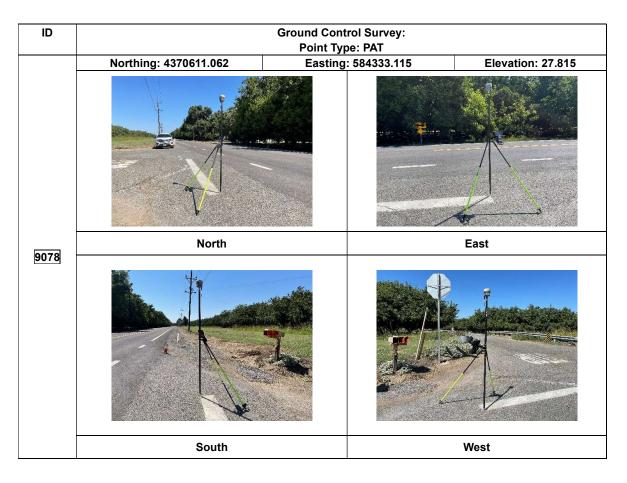


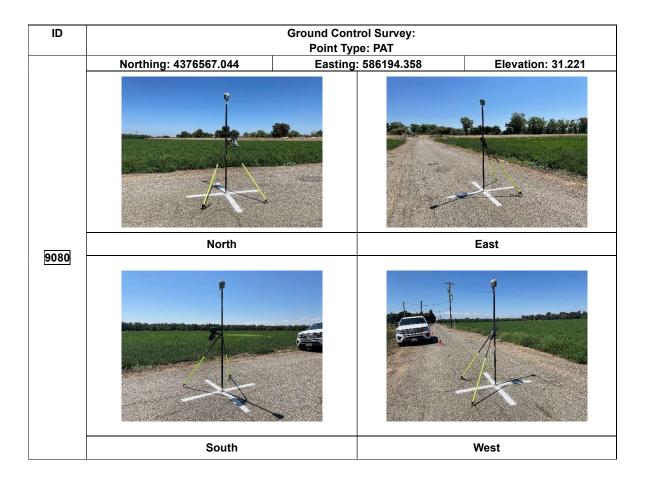


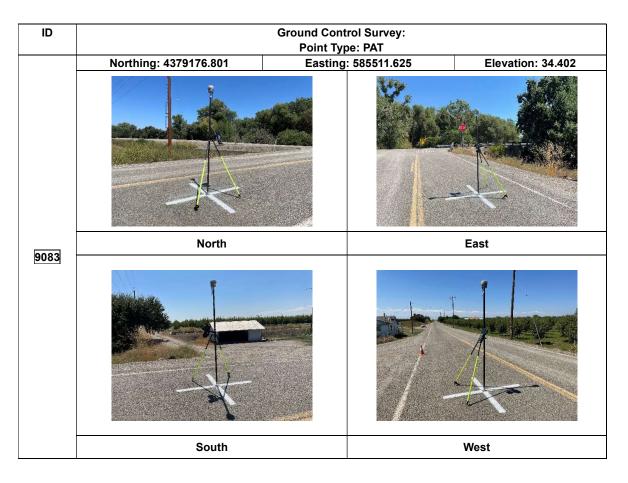












ID	Ground Control Survey:		
	Northing: 4254512.344	Point Type: PAT Easting: 628022.732	Elevation: 9.756
20000	North		East
	South		West

ID	Ground Control Survey: Point Type: PAT		
	Northing: 4259698.710	Easting: 628305.442	Elevation: 3.485
20002	North	East	
	South	West	

